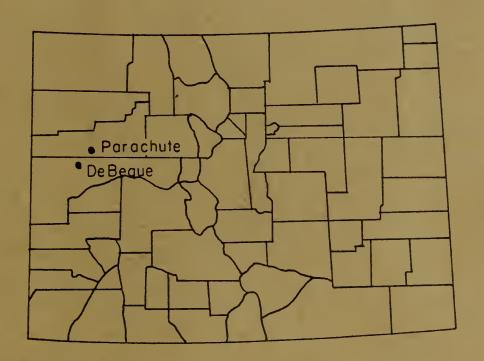
Historic, Archive Document

Do not assume content reflects current scientific knowledge, policies, or practices.

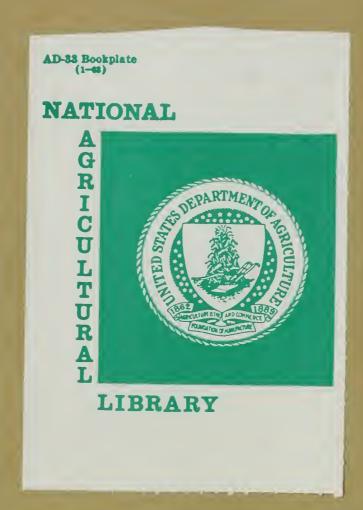


FLOOD PLAIN MANAGEMENT STUDY

PARACHUTE CREEK IN THE VICINITY OF PARACHUTE, CO. AND ROAN CREEK IN THE VICINITY OF DEBEQUE, CO.



Prepared by the
U.S. Department of Agriculture
Soil Conservation Service
Denver, Colorado
in cooperation with the
Colorado Water Conservation Board
Towns of Parachute and DeBeque
Garfield and Mesa Counties, Colorado
August 1985



PREFACE

CATALUGING = LP.

This report includes information on the flood hazard areas along Parachute Creek in the vicinity of Parachute Colorado and Roan Creek in the Vicinity of DeBeque, Colorado.

Because of the potential flood damages, detailed flood hazard studies have been recognized as an essential item in guiding the use of flood plains. The purpose of this report is to provide adequate mapping and data for implementing flood plain management programs.

Included in the report are information on past floods, flood potential, maps, profiles, cross sections, discharge data, and recommendations for reducing potential flood damages.

The Soil Conservation Service conducted the technical studies and prepared the report. These services were carried out in accordance with the Plan of Work of December 1982.

The assistance and cooperation provided by the Colorado Water Conservation Board, Towns of DeBeque and Parachute, and Garfield and Mesa Counties are appreciated and gratefully acknowledge. Financial assistance provided by the Board, the Towns and Counties included funds for photogrammetric maps, and cross section data. Three sheets of mapping (sheets 12, 13, 14) provided by Getty Oil Company were used along the upper reach of Roan Creek. The use of these maps is greatly appreciated.

The survey, hydrologic, hydraulic, and other pertinent data and computations are on file with the U.S. Department of Agriculture, Soil Conservation Service, 2490 West 26th Avenue, Denver, Colorado 80217,

telephone (303) 964-0295. Additional copies of this report may be obtained from the Colorado Water Conservation Board, or the Soil Conservation Service.

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FLOOD PLAIN MANAGEMENT STUDY PARACHUTE CREEK AND ROAN CREEK COLORADO

INTRODUCTION

This flood plain management report was prepared by the U.S.

Department of Agriculture, Soil Conservation Service, in cooperation with
the Colorado Water Conservation Board, Towns of Parachute and DeBeque,
Garfield and Mesa Counties. Interpretations of the flood plain management
study and recommendations to reduce damages are included; however, it is
beyond the scope of this report to provide specific proposals or plans to
rectify the flooding problems.

Objectives

The objective of this study is to provide flood plain management information and mapping to the towns of Parachute and DeBeque, Garfield and Mesa Counties for use in implementing flood plain management programs which will minimize potential flood losses. Included in the report are engineering and hydrologic data which will facilitate the development of a flood plain management plan, road and bridge plans and design, and non-structural and/or structural flood control measures (if needed).

Authority

This study was requested by the towns of Parachute and DeBeque, and Garfield and Mesa Counties through the Colorado Water Conservation Board (CWCB). The CWCB is the state coordinator for all flood plain information studies and is responsible for setting priorities and scheduling these studies. The CWCB and the Soil Conservation Service entered into a Joint Coordination Agreement for flood hazard analyses in January 1972

(revised November 1978). The Plan of Work for the Study was prepared in March, 1982.

Section 37-60-106(1)(c), Colorado Revised Statutes, authorizes the Colorado Water Conservation Board "to designate and approve storm or floodwater runoff channels or basins, and to make such designations available to legislative bodies of cities and incorporated towns, to county planning commissions, and to boards of adjustment of cities, incorporated towns, and counties of this state." The Board provides assistance to local governments in development and adoption of effective floodplain ordinances. In addition, the Board will provide technical assistance to local entities during the performance of floodplain information studies within Colorado. Presently, financial assistance for the performance of floodplain studies is no longer available from the board.

Section 30-28-111 for county governments and Section 31-23-201 for municipal governments of the Colorado Revised Statutes, states: The cities, incorporated towns, and counties within the study area may provide zoning regulations: "...to establish, regulate, restrict, and limit such uses on or along any storm or floodwater runoff channel or basin that has been designated and approved by the Colorado Water Conservation Board, in order to lessen or avoid the hazards to persons and damage to property resulting from the accumulation of storm or floodwaters..."

Therefore, upon official approval of this report by the Colorado

Water Conservation Board, the areas described as being inundated by the

100-year flood can be designated as flood hazard areas and their use regulated accordingly by the local agencies.

Flood plain management studies are carried out by the Soil

Conservation Service as an outgrowth of the recommendations in <u>A Report by</u>

the Task Force on Federal Flood Control Policy, House Document No. 465

(89th Congress, August 10, 1966), especially Recommendation 9(c),

Regulation of Land Use, which recommended the preparation of preliminary reports for guidance in those areas where assistance is needed before a full flood plain information report can be prepared or were a full report is not scheduled.

Authority for funding flood plain management studies is provided by Section 6 of Public Law 83-566, which authorizes the U.S. Department of Agriculture to cooperate with other federal, state and local agencies to make investigations and surveys of the watersheds and rivers and other waterways as a basis for the development of coordinated programs. In carrying out flood plain management studies, the Soil Conservation Service is being responsive to Executive Order 11988, entitled "Flood Plain Management", and Executive Order 11990, entitled "Protection of Wetlands" (both effective May 24, 1977).

DESCRIPTION OF THE STUDY AREA

Basin Characteristics

This study involves the Roan Creek Basin (507.5 square miles) and Parachute Creek Basin (200 square miles). They are adjacent drainages, both flowing in a southeasterly direction into the main stem of the Colorado River.

The Basins headwaters originate on the Roan Plateau at elevations near 8800 ft. The Roan Creek Channel empties into the Colorado River at an elevation of about 4900 ft. near the town of Debeque. Parachute Creek flows into the Colorado River upstream from Roan Creek at an elevation of 5050 ft., near the town of Parachute.

The topography is primarily plateau-like table land deeply cut by watercourses which form steep canyons. The region is rich in coal and oil shale deposits.

The climate of the area is influenced by Pacific storm systems that move from west to east. The nearest national weather station, representative of the lower part of the basin, is at Rifle. The mean annual temperature at Rifle is 47°F with about 109 days of growing season between the spring and fall 32°F frost occurrences. The mean annual precipitation at Rifle is just over 11 inches. This increases to about 20-25 inches at the higher elevations. Wintertime precipitation is usually snow from late October to early April.

The soils include Mollisols at the higher elevations in the Upper Basin and Aridisols and Entisols in the lower part of the Basin.

Fluvents are dominant adjacent to stream channels. The geologic formations include the Tertiary Green River, which contains oil shale deposits, and the Wasatch.

The higher ridges have conifer and aspen while lower areas have juniper and pinyon interspersed with sagebrush. Willows and cottonwoods grow along the streams. There is also some irrigated cropland at lower elevations along the streams.

The towns of Parachute, on Parachute Creek, and Debeque, on Roan Creek, are the only communities in the basins. They are situated adjacent to the main channels near their confluences with the Colorado River.

Study Limits

The area of study includes the flood plain of Parachute Creek from its confluence with the Colorado River upstream to the west edge of Section 28, T6S, R96W (7.5 miles) and the flood plain of Roan Creek from its confluence with the Colorado River upstream to the Garfield County Line (5.2 miles).

The towns of Parachute and Debeque are within the study area and parts of each community are within the flood plains.

Natural and beneficial Flood Plain Values

The flood plains along Roan and Parachute Creeks contain areas of irrigated pasture and hayland interspersed with areas of natural vegetation. Along the channel, the vegetation consists of a variety of forbs, grasses, sedges and rushes interspersed with cottonwoods, willows and siberian elm. The meandering channel, passing through cropped farmland, provides a diversity in landscape.

The lush vegetation in the flood plains makes a vivid contrast to the barren hills surrounding the general area. This diversity enhances the visual aestheties and wildlife habitat values in the area.

These flood plains support a variety of wildlife species such as: mule deer, coyote, cottontail, red-winged blackbird, blue herron, song

sparrow, black-headed grosbeak, red-tailed hawk, golden eagle, bald eagle, Canada goose, mallard and many other species of wildlife. These riparian areas are very important in the arid regions of Colorado. The proximity to water and robust vegetation supported by the water regime attract more species of wildlife to this habitat type than any other in western Colorado.

RELATED FLOOD STUDIES

The Morrison-Knudsen Company, Inc. carried out a study to provide Chevron Shale Oil Company with a preliminary 100-year flood boundary map on Roan Creek. It was intended as a guide for DeBeque's expansion plans. The report is dated July 1981 titled "Clear Creek Shale Oil Project

The Corps of Engineers, Sacramento District, published an Internal Official Memorandum Report "Flood Insurance Study Hydrology, Garfield and Mesa Counties, Colorado" dated November 1975. This was a relatively broad study that included drainage area vs. cubic feet per second per square mile envelope curves. The curves were intended for estimating peak discharges on streams in Garfield and Mesa Counties, Colorado, apparently for flood insurance purposes.

FLOOD HISTORY

Major flooding along Parachute Creek and Roan Creek is caused by rapid melting of the mountain snowpack during late May to early July as well as summer rainstorms. There is also the potential for flooding as a result of rainfall occurring on melting snow. The snowmelt floods are characterized by moderate peaks, large volumes, and long durations. The summer floods have characteristics of high peaks, and short flow durations.

The most recent damaging flood occurred during the end of May, 1983 on Roan Creek. It resulted from the melting of an unusually deep snowpack. The above average spring runoff caused concern all along the Colorado River, including the large Glen Canyon Dam. People in the area of Roan Creek reported that water started rising about May 15 and remained at a high stage until June 6.

About 10 families along the Creek were affected. Irrigation systems were washed out making it impossible for local ranchers to irrigate.

Approximately 200 acres of irrigated pasture and hayland were damaged with debris, sediment, scarring, and streambank erosion. About 1/2 of the total crop production in the Roan Creek Drainage was damaged, along with 12 miles of county road and 3 bridges. Total damages were estimated at over 1 million dollars. Streamgages on Roan Creek were discontinued in 1982 therefore, the peak discharge for this flood was not determined.

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Other events of significance on Roan Creek include:

May 11, 1980 - discharge = 2,020 cfs

May 19, 1979 - discharge = 1,190 cfs

Sep 19, 1972 - discharge = 1,900 cfs

Aug 20, 1971 - discharge = 1,000 cfs

May 26, 1967 - discharge = 1,220 cfs

May 21, 1922 - discharge = 1,110 cfs

Significant flood events on Parachute Creek include:

May 18, 1979 - discharge = 944 cfs

Aug 19, 1977 - discharge = 2,310 cfs
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Jul 31, 1976 - discharge = 2,600 cfs
May 5, 1952 - discharge = 912 cfs
May 20, 1922 - discharge = 795 cfs

low records are far from complete on Parachute Cro

Streamflow records are far from complete on Parachute Creek and Roan Creek, therefore a number of other flood events no doubt have occurred without being recorded.

INVESTIGATIONS AND ANALYSIS

Interpretation and Use of Report

A. Frequency and Discharge

The 10-, 50-, 100-, and 500-year flood events are used as the flood frequencies for this flood plain analysis. Thus the data developed in this report will be compatible not only for regulation purposes, and H.B. 1041 designation but also for Federal Insurance Administration flood insurance rate studies.

These various flood events have an average occurrence of once in the number of years as indicated. For example, the 100-year flood occurs, on the average, once in a 100 year period, and has a one percent chance of being equaled or exceeded in any given year.

The particular uses for the various flood events in addition to those stated above are as follows:

10-Year and 50-Year Flood Events

Information regarding these lower frequency floods is especially useful for future engineering studies and land use planning purposes related to minor road systems, minor channel improvements, the location of parks and recreational facilities, agricultural lands, and appurtenant structures. The use of the lower frequencey floods may be considered in planning flood prevention projects to protect agricultural areas, or other property where risk to life is not a factor.

100-Year Flood Event

The 100-year flood event may be used in lieu of lower frequencies for engineering design purposes where greater security from structure failure is desired.

However, the most important use of the 100-year flood event lies in flood plain management and land use planning as set forth in the state statutes. The State of Colorado considers the 100-year frequency flood as the flood event to be used in designing and protecting structures and dwellings for human occupation. Therefore, all flood plain regulations are based upon the 100-year flood.

500-Year Flood Event

The 500-year flood event is useful in making the public aware that floods larger than the 100-year flood can and do occur. Just because a person is living above the 100-year flood boundary does not mean that he is completely safe from flooding. The 500-year flood event can also be used for regulating high risk developments within the flood plain such as nuclear power plants, or the storage or manufacture of toxic or explosive materials.

B. Flood Elevation

The included exhibits and tables display study results. Flood crest elevations for the 10-, 50-, 100-, and 500-year floods, as determined at each cross section, may be found in Table 1 "Flood Frequency-Elevation and Discharge Data". The Cross Section Exhibits, B-1 through B-8, show a graphical representation of high water elevations at typical valley cross sections. Water surface elevations computed at each cross section were used to prepare flood profiles, exhibits A-1 through A-16, which show the streambed elevation in relation to high water elevations for the 10-year 50-year, 100-year, and 500-year frequency floods.

The flood profiles may be used in areas where controversy arises over the 100-year flood boundary shown on the Flood Plain Maps. Since the flood profile exhibits give the water surface elevation at a specific

point on the reference line, the high water elevations can be surveyed on the ground to alleviate any discrepancies on the base map.

C. Flooded Areas

Flood plain maps, sheets 1 through 14, show the boundary of the 100-year and 500-year flood plains. The flood plain boundaries were plotted from the flood profiles by determining channel stationing of flood contours at the same interval as the topographic maps. Flood contours, shown as wiggly lines, extend perpendicular to the direction of flow and intersect the ground at the edge of the flood plain.

The areas included within the flood line boundaries are about 550 acres for the 100-year frequency and 730 acres for the 500-year.

Upon official approval of this report by the Colorado Water

Conservation Board, the area outlined by the 100-year flood boundary may
be regulated accordingly by the local officials.

D. Floodway

Encroachments on flood plains, such as artificial fill, can reduce the areal extent of a flood plain if provisions are made for increased flood heights. As an alternative to the present flooding situation, a possible floodway with flood plain encroachment was analyzed in this study. This was simply a hydraulic analysis in which the flood plain was theoretically modified to contain flooding within selected encroachment boundaries. The resulting effects on flood elevations are shown in an Appendix separate from this report.

Hydrology

Tributaries to the Colorado River in the vicinity of DeBeque to Glenwood Springs, Colorado are streams that flood from snowmelt as well as from summer rain. The intent of this analysis was to separate the annual peak discharges into rainfall events and snowmelt events. Separate frequency curves should be combined statistically to produce a final discharge frequency curve. There was insufficient streamflow data of rainfall flood events to accomplish this, therefore the SCS TR-20 computer program was used to simulate rainfall flood peaks. The model was used on 16 watersheds of varying sizes. A regional curve of drainage area vs. peak discharge and frequency was developed for rainfall flooding. The TR-20 analysis included the standard SCS Type II (24 hour) rainfall distribution and curve numbers for an average antecedent soil moisture condition (AMC-II). The discharge vs. drainage area data from this analysis were plotted, and a regression line computed for several frequencies.

A regional curve was developed for snowmelt flood events using streamgage data from 8 streamgages in the area. The Log Pearson III frequency distribution (as defined in WRC Bulletin 17-B) was used with a regional skew weighted with each computed station skew. The data was plotted and frequency lines drawn.

The two regional discharge frequency-drainage area curves (rainfall and snowmelt) were combined using a standard probability equation:

P(comb)=P(snow)+P(rain)-(P(snow)xP(rain)).

This combined regional curve is proposed for studies along the Colorado River Tributaries in the vicinity of Debeque to Glenwood Springs, Colorado.

Hydraulics

The U.S. Army Engineers HEC-2 computer program was used to perform water surface profile computations. Several bridges and culverts exist along the channels through the study reach. Dimensions for these road crossings were determined from field investigations and the data was integrated into appropriate cross section data.

Cross section data, and reach length information were obtained from photogrammetric maps. Maps were prepared especially for this study, except for the upper reach of Roan Creek. Available maps provided by Getty Oil Company were used for this upper reach. The Getty Oil maps have 5.0 ft. contour intervals whereas the other maps have 2.0 ft. intervals.

Hydraulic roughness coefficients (n-values) were determined from field investigations and documented with photographs (in technical addendum). A tabulation of roughness coefficients is included in the technical addendum for various locations along the study reach.

Water surface profiles, typical cross sections and maps showing the 100-year and 500-year flood lines are shown on included exhibits and flood plain maps. Table 1 shows computed flood elevations at specific cross sections.

Flood lines were located on the maps by transferring flood elevations (at map contour intervals) from plotted profiles (from HEC-2) to the maps, using stationing along the main channel as the location reference. These points were connected and smoothed to create the map flood lines.

Significant divided flow occurs at the railroad crossing southwest of Parachute on Parachute Creek. This divided flow occurs when restricted flow from the railroad bridge overtops the tracks for the 100-year and

500-year frequency discharges. This overflow moves in a northeast direction away from the channel of Parachute Creek. An approximate location of the path of this overflow is shown on included flood plain maps. Flood profiles for this segment of flow are now shown in this report.

FLOOD PLAIN MANAGEMENT

Potential flood damages to existing development and possible loss of life can be alleviated or lessened through non-structural and structural methods.

Non-structural methods include: flood plain regulations, land treatment, flood warning and forecasting systems, flood insurance, flood proofing, and flood fighting and emergency evacuations.

Local Regulations

The need to minimize property damage due to flooding has been recognized by planners and local community officials. Subdividers and developers are required to submit proposed storm drainage plans to the planning commission for approval. In the past, drainage plans have been prepared signularly or on a plat-by-plat basis. Information contained in this report will be useful in developing a master drainage plan for the study area. This report provides the outline of flood hazard areas on large scale maps specifically for this purpose.

The city may provide zoning regulations...

... "to establish, regulate, restrict, and limit such uses on or along any storm or floodwater runoff channel or basin, as such storm or floodwater runoff channel or basin has been designated and approved by the Colorado Water Conservation Board, in order to lessen or avoid the hazards to persons and damage to property resulting from the accumulation of storm or floodwaters"...

as stated in Section 30-28-111 for county governments and Sections 31-23-201 for municipal governments of the Colorado Revised Statutes 1973.

Colorado Natural Hazard Area Regulations

In 1974, the Colorado General Assembly passed House Bill 1041, a bill "concerning land use, and providing for identification, designation, and administration of areas and activities of State interest,..." (H.B. 1041, Title 24, Article 65.1, CRS, as amended). Areas of State interest include natural hazard areas, or those areas that are "so adverse to past, current, or foreseeable construction or land use as to constitute a significant hazard to public health and safety or to property." Flood plains are natural hazard areas.

With reference to the administration of natural hazard areas, section 24-65.1-202(2)(a) of the Act provides: Flood plains shall be administered so as to minimize significant hazard to public health and safety or to property; open space activities shall be encouraged; structures shall be designed in terms of use and hazards; disposal sites and systems shall be protected from inundation by floodwaters; and activities shall be discouraged which, in time of flooding, would create significant hazards to public health and safety or to property.

The Act further provides that after promulgation of guidelines for land use in natural hazard areas..., the natural hazard areas shall be administered by local government in a manner which is consistent with the guidelines for land use in each of the natural hazard areas.

Colorado Water Conservation Board Designations

Concerning the designations of flood plain, the Colorado Water Conservation Board is charged with the primary responsibility for:

- 1. Making recommendations to local governments and the Colorado Land
 Use Commission.
- 2. Providing technical assistance to local governments.

The Board's power and duty is ...

..."to devise and formulate methods, means and plans for bringing about the greater utilization of the waters of the state and prevention of flood damages therefrom, and to designate and approve storm or floodwater runoff channels or basins, and to make such designations available to legislative bodies of cities and incorporated towns, to county planning commissions, and to boards of adjustment of cities, incorporated towns, and counties of this state"...

as stated in Section 37-60-106 (1) (c) of the Colorado Revised Statutes

Upon review and approval of this report, the Colorado Water

Conservation Board will designate and approve as flood plain areas those

areas inundated by the 100-year flood as described by the floodwater

surface elevations and profiles in this report. The use of the designated

flood plain areas may then be regulated by the local government.

Model Regulations

In the model flood plain regulations, adopted by the Colorado Water Conservation Board, the statement of purpose is to promote the public health, safety, and general welfare, and minimize flood hazards and losses by provisions designed to:

- Promote sound planning and land use, and permit only such uses within flood plains that will not endanger life, health, and public safety or property in times of flooding.
- 2. Protect the public from avoidable financial expenditures for flood control projects, flood relief measures, and the repair and restoration of damaged public facilities.
- 3. Prevent avoidable interruption of business and commerce;
- 4. Minimize victimization of unwary home and land purchasers; and

5. Facilitate the administration of flood hazard areas by establishing requirements that must be met before use or development is permitted.

The Board's model flood plain regulations offer two options for management of the 100-year flood plain. These are the Hazard Area Concept and the Floodway Concept.

The Hazard Area concept defines the areas of the flood plain in which waters of the 100-year flood attain a maximum depth greater than one and one-half feet as a high hazard areas, and a depth less than this as a low hazard area.

The Board recommends that no basements should be allowed for structures located within the low hazard area and all habitable living quarters (first floors) should be constructed a minimum of one foot above the 100-year floodwater surface elevations. Development is prohibited in high hazard areas.

The Floodway concept defines the channel of a stream and adjacent flood plain areas that must be kept free of development in order to safely pass the 100-year flood with a minimal rise in the water surface elevation. The rise must be no more than one foot to meet federal and state standards.

There are several methods used in floodway computations. One such theoretical method is computed on the basis of equal conveyance reductions for each side of the flood plain. A rise concept floodway was computed during this study. Because of the large amount of computional data, floodway information and data are included in appendix II separate to this report. Data are in tabular form and include floodways widths,

cross sectional flow area, and average velocities. Computations are for an increase in rise of water surface elevations in 0.5' increments from 0.0' to 1.5' above the 100-year flood.

Flood Insurance

The National Flood Insurance Act of 1968 (Title XIII of the Housing and Urban development Act, P.L. 90-448) recognized the necessity for flood plain management. This Act makes federally subsidized insurance available to citizens in communities that adopt regulations controlling future developments of their flood plain. In respect to encroachment on the flood plain, the regulations require:

New residential construction or substantial improvement of existing homes must have the lowest floor level above the elevation of the 100-year flood.

Non-residential construction must meet the same standard or be flood proofed to that level.

The 1968 Act benefits owners of structures already in the flood-prone areas by providing insurance coverage that had been unavailable through private companies. The Act created a cooperative program of insurance against flood damage by the private flood insurance industry and the federal government.

The amount of coverage available and the premium rate varies considerably depending on property location within the flood plain and the property value. All property owners shown in this study to be within areas subject to flooding should consider the purchase of flood insurance.

Additional information on the flood Insurance Program is available from local insurance agents or brokers and the:

Federal Emergency Management Agency

Division of Insurance and Mitigation

Building 710

Denver Federal Center

Denver, Colorado 80225

Telephone 235-4830

The National Flood Insurance Program uses the floodway concept in its' rate studies for communities participating in its' regular programs. Flood Warning and Flood Forecasting Systems

The National Oceanic and Atmospheric Administration (NOAA) through its' National Weather Service (NWS), maintains year-around surveillance of weather and flood conditions. Daily weather forecasts are issued through the NWS and disseminated by radio and television stations. A general alert to the danger of flash flooding is one of the services provided by the National Weather Service.

Evacuation Plan

An "Emergency Evacuation and Operations Plan" would provide for alerting the public of potential flooding, and coordinating community and county services during an emergency. Plan implementation during the time of an emergency requires cooperation of the general public as well as local officials. This is especially important for flood fighting, evacuation, and rescue operations. Communication is extremely important during flood alerts. Warnings issued through the National Weather 21

Service are disseminated by radio to state and local officials.

Structural Flood Control Measures

Under present conditions, bridges along the lower reaches of Parachute Creek and Roan Creek restrict flow and contribute to out-of-channel flooding. The most critical constriction is located at cross section 235 (Denver and Rio Grande Railroad Bridge) at the southern edge of Parachute. The bridge constriction causes about 22 percent of the 100-year flood discharge to overflow the tracks and flow overland in a southeasterly direction towards the Colorado River.

If this bridge could be enlarged to accommodate the 100-year discharge without causing excessive backwater effects, the flood elevation at First Street would be lowered, by 2.2 ft. to an elevation of 5,095.2 (see figure 1). Some additional reduction in flood elevation could be achieved if other bridges such as Interstate 70 and First Street were also enlarged. These bridge constrictions contribute to severe imundation of a large portion of the town upstream of these crossings

The railroad bridge on Roan Creek (Sec 139) also causes significant backwater effect. If this bridge were enlarged to accommodate the 100-year discharge, the flood elevations at Section 140, midway between the railroad and Road 44, would be lowered by 2.7 ft. to an elevation of 4,906.5. However, the hydraulic affect of an enlarged railroad bridge would essentially be gone at Road 44 (see figure 2). Any reduction in flood elevations at this location and above would have to come from changes to the Road 44 bridge.

Other structural measures such as floodwater retarding dams could also alleviate flooding. This alternative was not studied herein because of the amount of time and data required.

RECOMMENDATIONS

The following recommendations are included for considertion in reducing potential flood damages.

- 1. Local units of government should implement a flood plain management plan.
- 2. Existing restrictions that contribute to overbank flooding should be corrected where possible and when possible.
- 3. Detailed studies of specific structural alternative measures such as floodways and dikes to reduce flooding should be considered.
- 4. Owners and occupants of buildings and other property within or adjacent to the delineated flood boundary should consider flood insurance.
- 5. Public information and education programs on flood hazards should be made available to the public.
- 6. Native habitat along the main channels should be maintained to preserve channel stability and provide wildlife habitat.

GLOSSARY OF TERMS

- Channel A natural or artificial water course of perceptible extent with definite banks to confine and conduct continuously or periodically flowing water. Channel flow is that water which is flowing within the limits of the defined channel.
- Flood Water from a river, stream, water course, lake or other body of standing water, that temporarily overflows the boundaries within which it is ordinarily confined.
- Flood Crest The maximum stage or elevtion reached by the waters of a flood at a given location.
- Flood Frequency A means of expressing the probability of flood occurrences as determined from statistical analysis of representative streamflow or rainfall and runoff records. The frequency of a particular stage or discharge is usually expressed as occurring once in a specified number of years. The 10-, 25-, 50-, 100- and 500-year frequency floods have an average frequency of occurrence in the order of once in the number of years as indicated.
- 10-Year Flood A flood having an average frequency of occurrence of once in 10 years. It has a 10 percent chance of being equaled or exceeded in any given year.
- 100-Year Flood A flood having an average frequency of occurrence of once in 100 years. It has a l percent chance of being equaled or exceeded in any given year.
- Flood Hazard Areas Areas susceptible to flood damage.
- Flood Peak The highest stage or discharge attained during a flood event; also referred to as peak stage or peak discharge.

- Flood Plain The relatively flat or lowland area adjoining a river, stream, watercourse, lake, or other body of standing water which has been or may be covered temporarily by flood water. For administrative purposes the flood plain may be defined as the area that would be inundated by the 100-year flood.
- Left Stream Bank The left bank of the stream when looking downstream.
- Perched Channel Flow A condition where the flow elevation in the outer portions of the flood plain is higher than the flow elevation in the main channel. This condition occurs when a higher secondary channel receives inflow from some location upstream and maintains a flatter slope than the main channel.
- Reach A hydraulic engineering term used to describe longitudinal segments of a stream or river.
- Right Stream Bank The right bank of the stream when looking downstream.
- Runoff That part of precipitation, as well as any other flow contributions, which appears in surface streams of either perennial or intermittent form.
- Stream Any natural channel or depression through which water flows whether continuously, intermittently, or periodically, including modification of the natural channel or depression.
- Structure Anything constructed or erected, the use of which requires a more or less permanent location on or in the ground. Includes but is not limited to bridges, buildings, canals, dams, ditches, diversions, irrigation systems, pumps, pipelines, railroads, roads sewage disposal systems, underground conduits, water supply systems and wells.
- Typical Valley Cross Section An engineering drawing of a vertical section of a stream channel and adjoining landscape as viewed in a downstream direction. The drawing represents a specified location

within a designated stream reach.

- Water Surface Profile (This term is synonymous with Flood Profile) a graph showing the relationship of the water surface elevation of a flood event to location along a stream or river.
- <u>Watersheds</u> A drainage basin or area which collects runoff and transmits it usually by means of streams and tributaries to the outlet of the basin.

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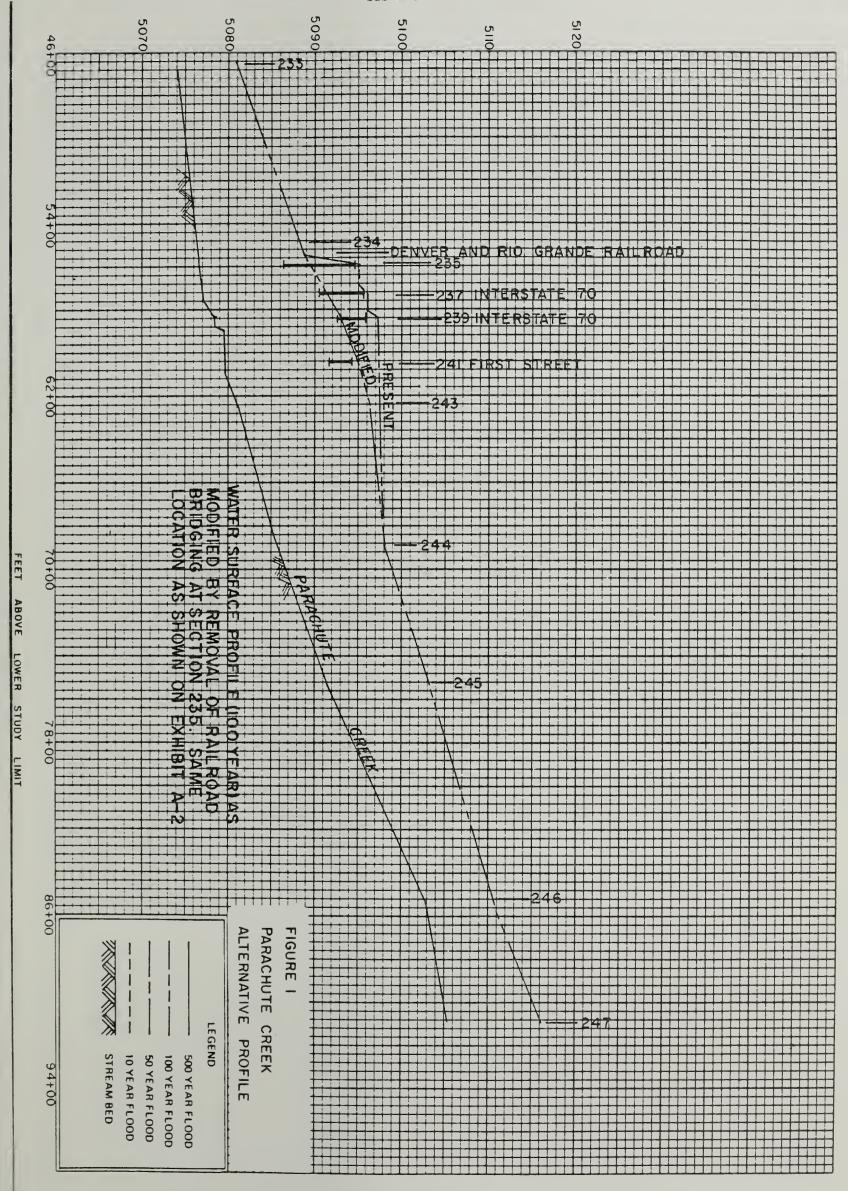
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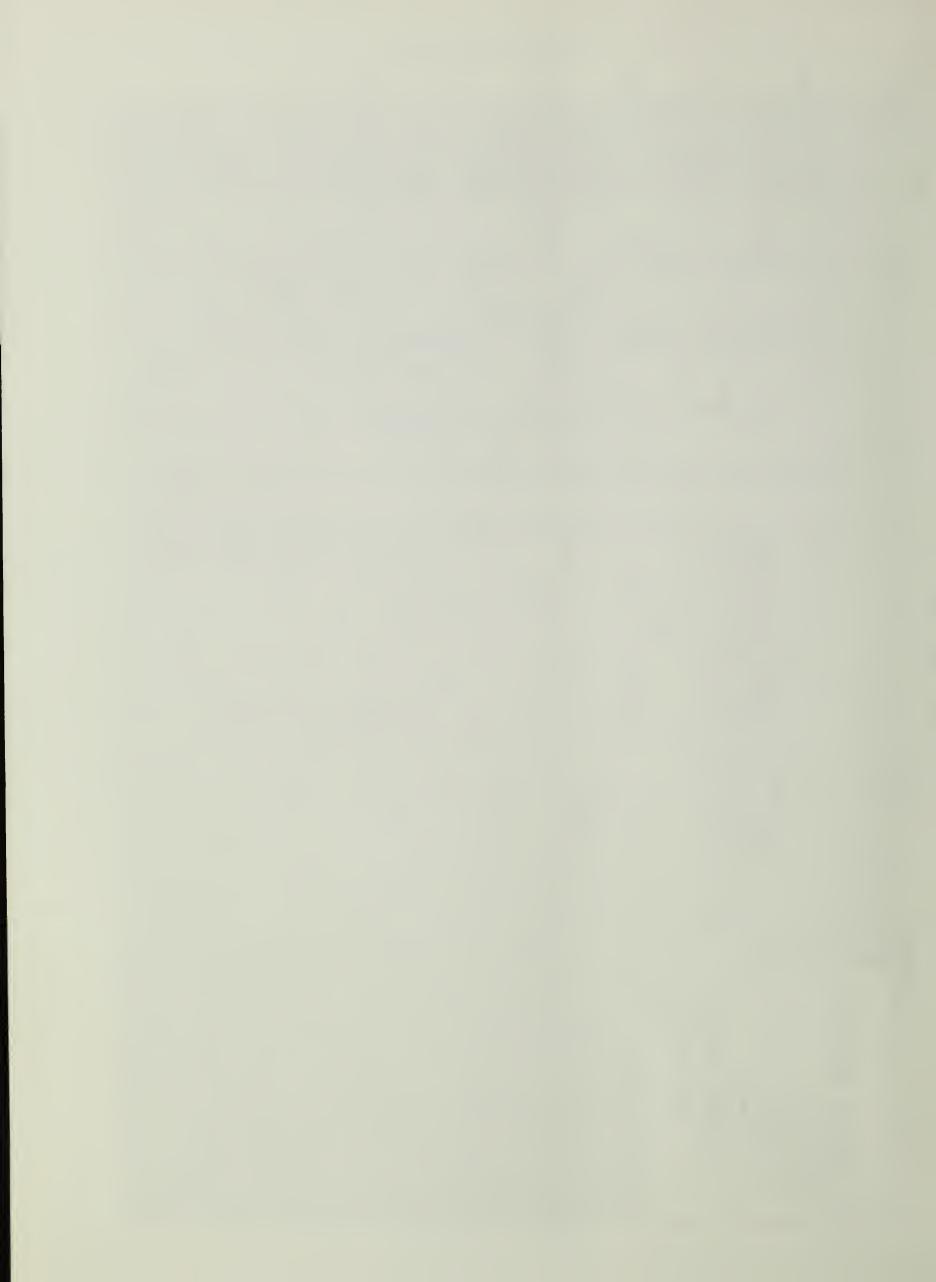
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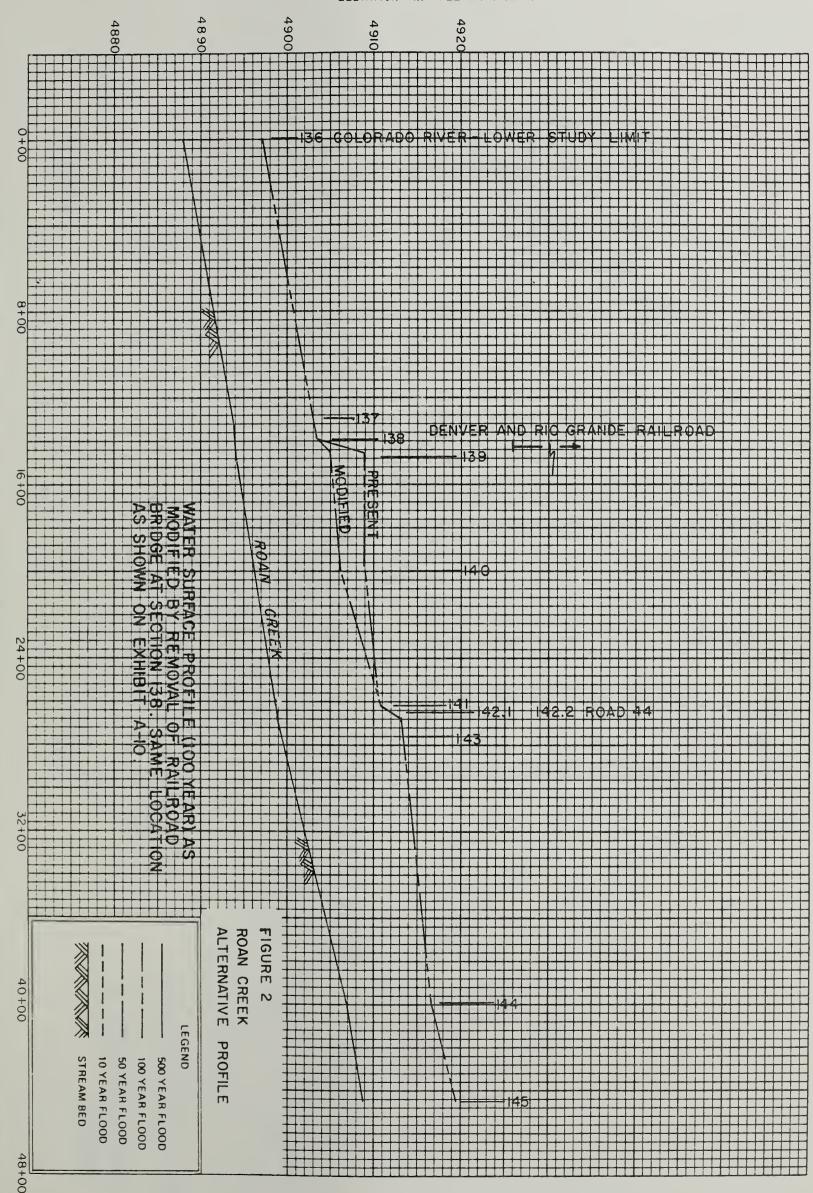
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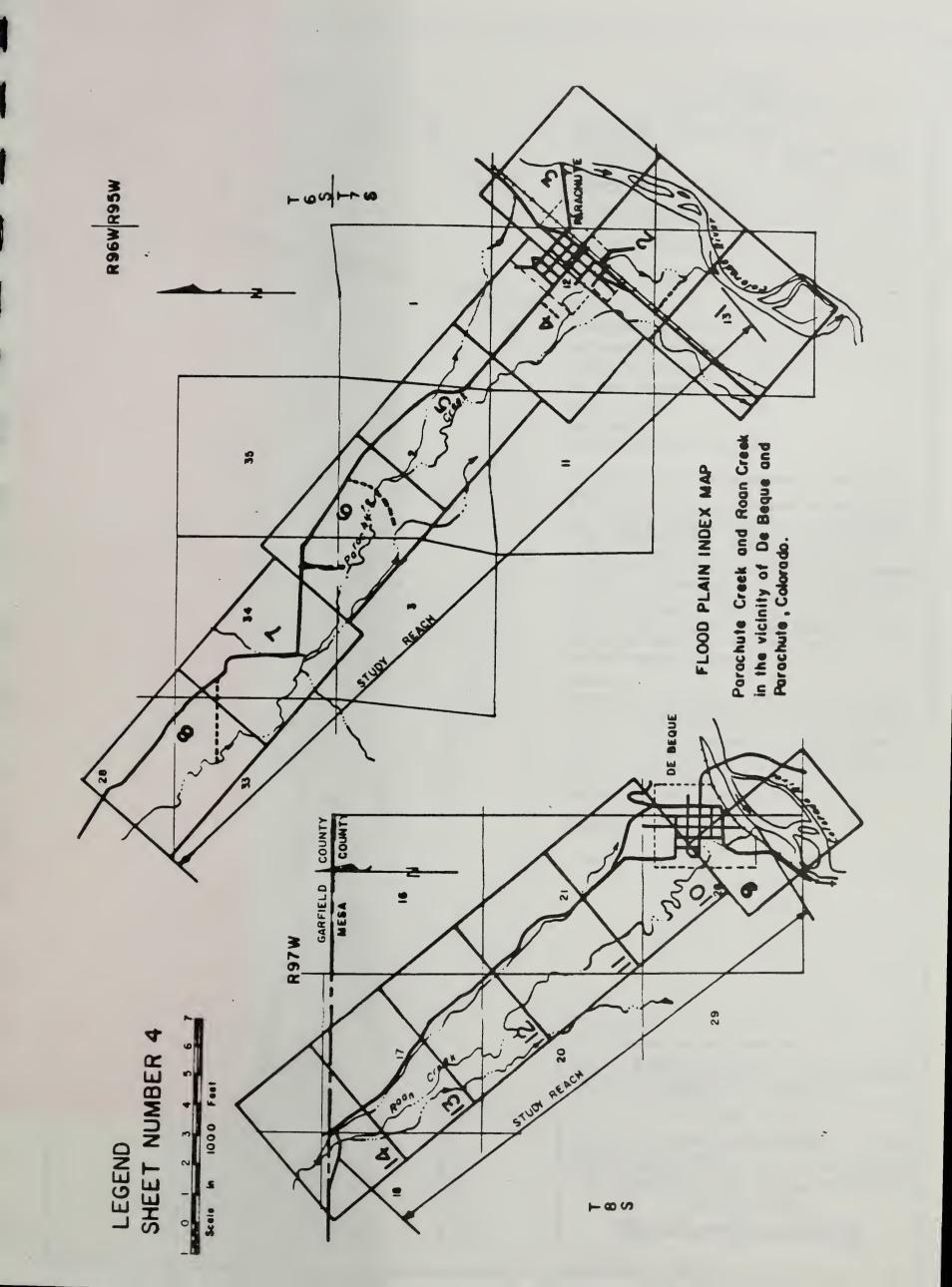
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LOWER

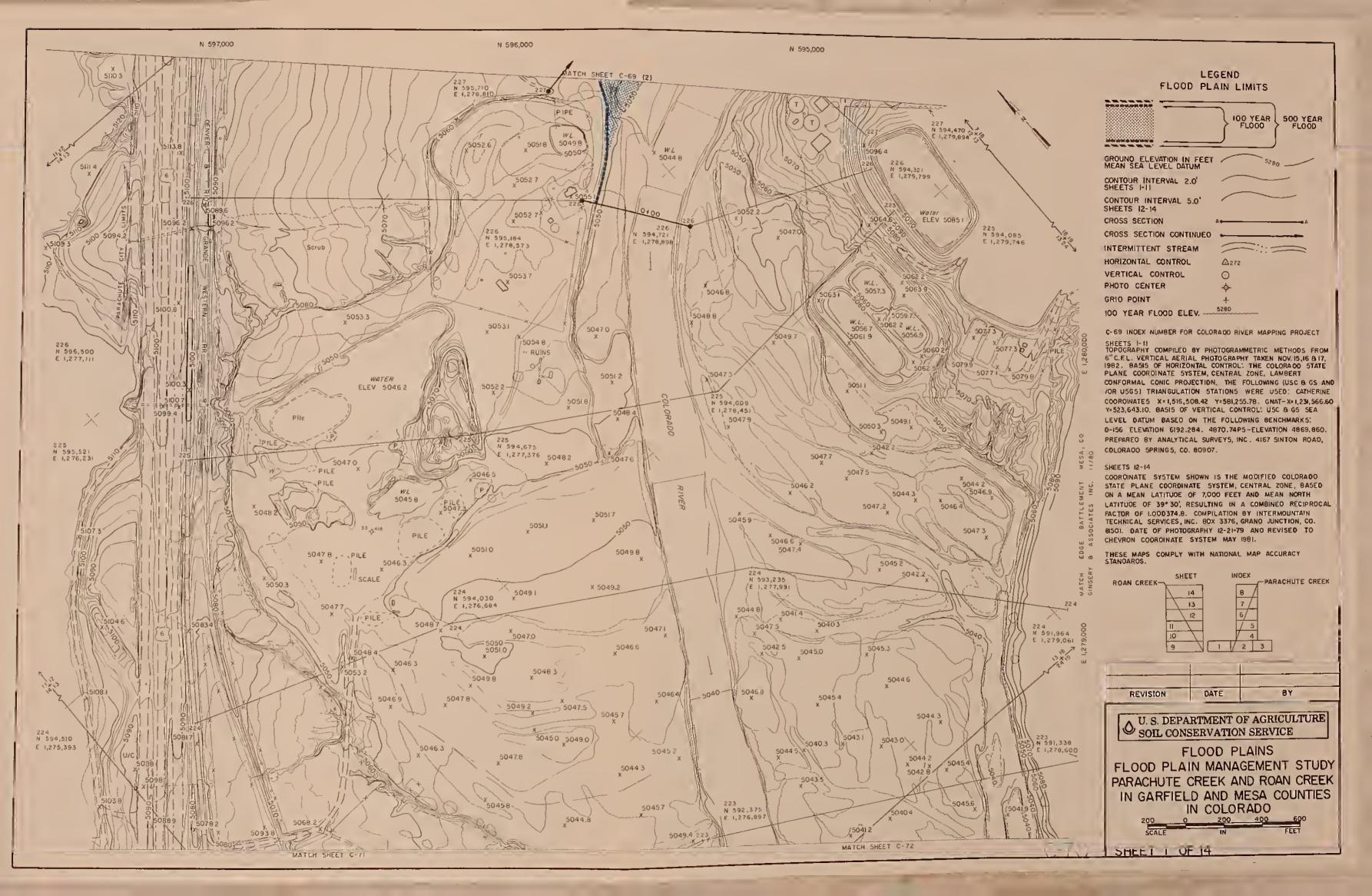
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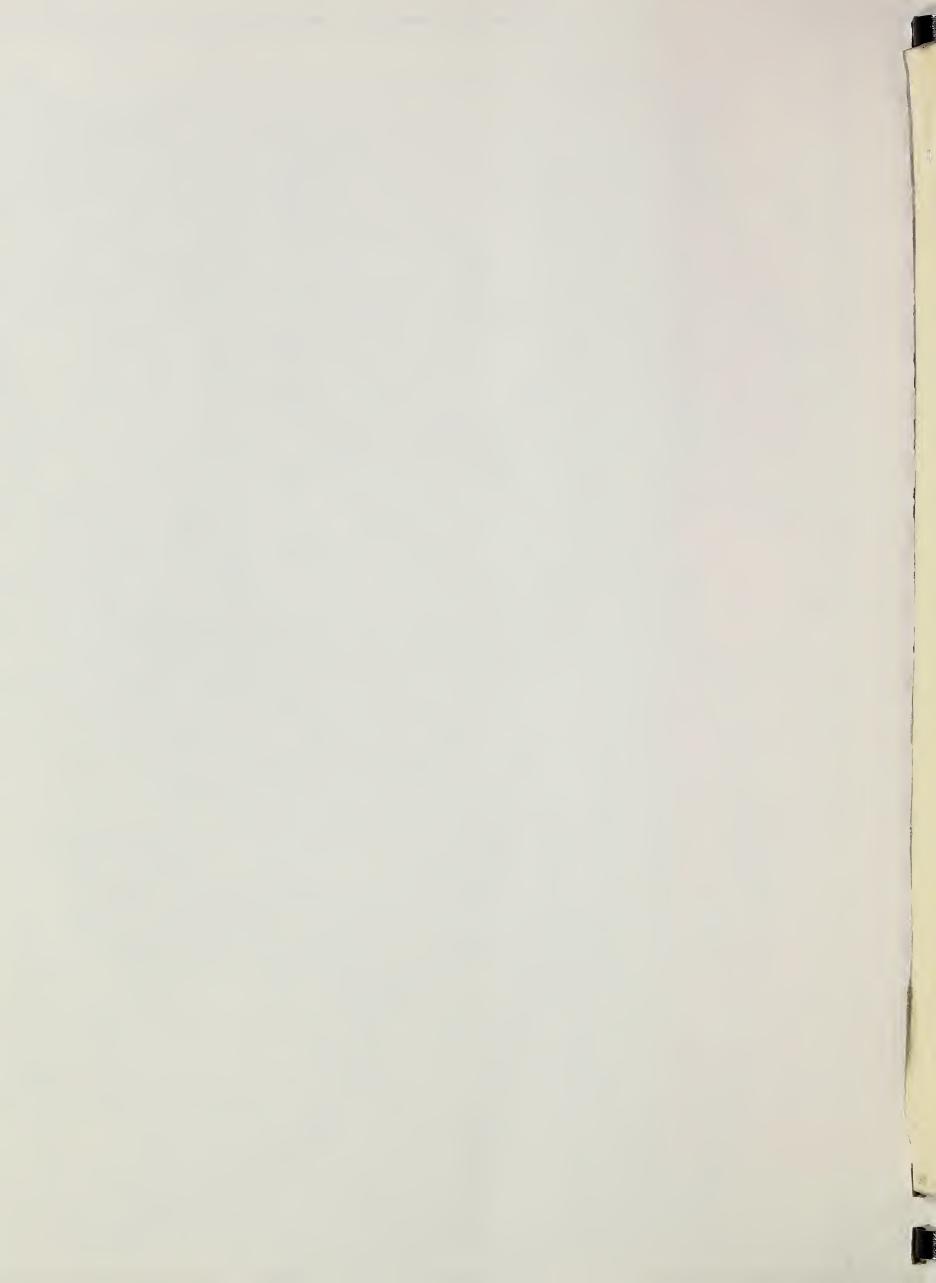
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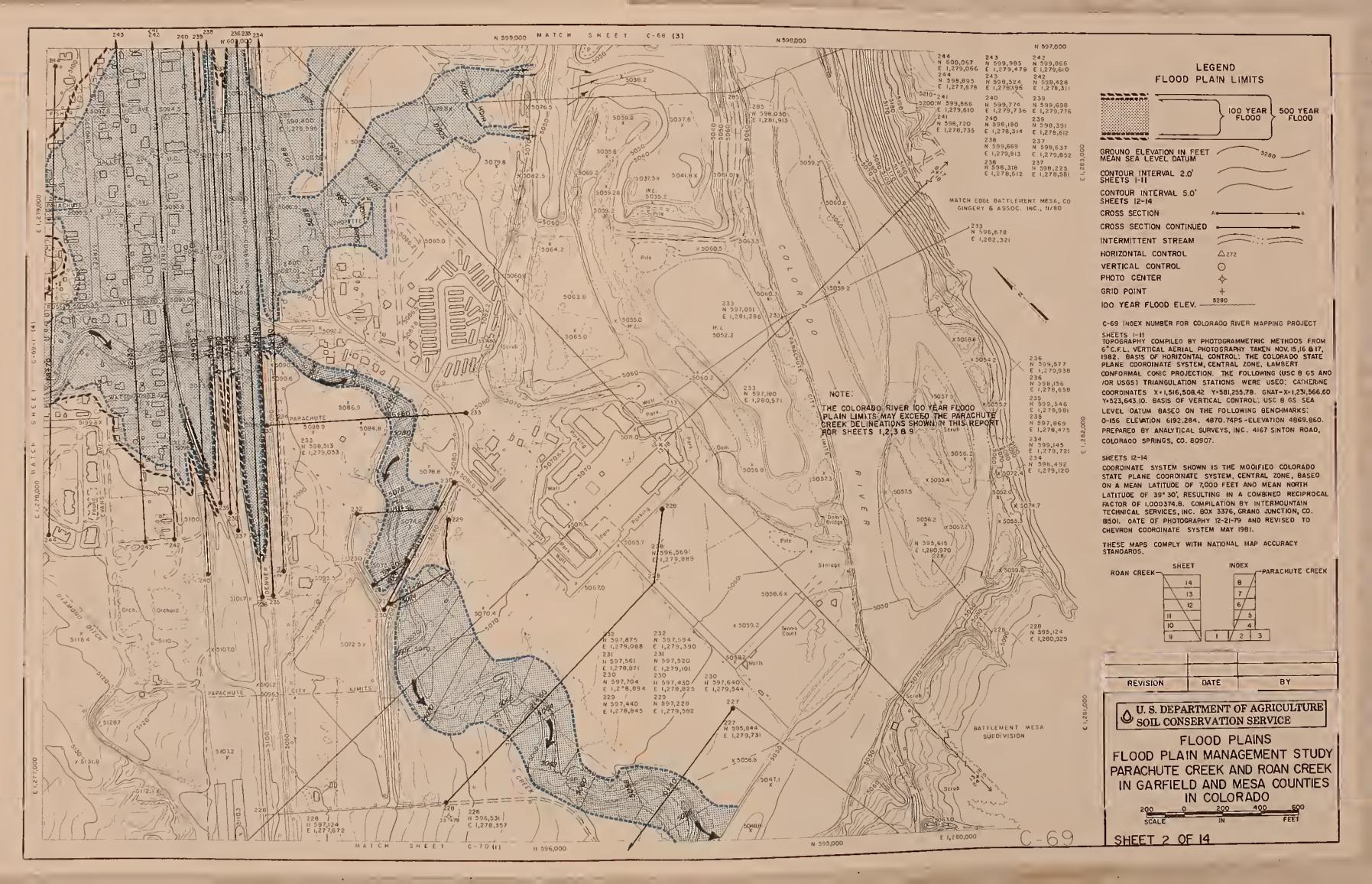


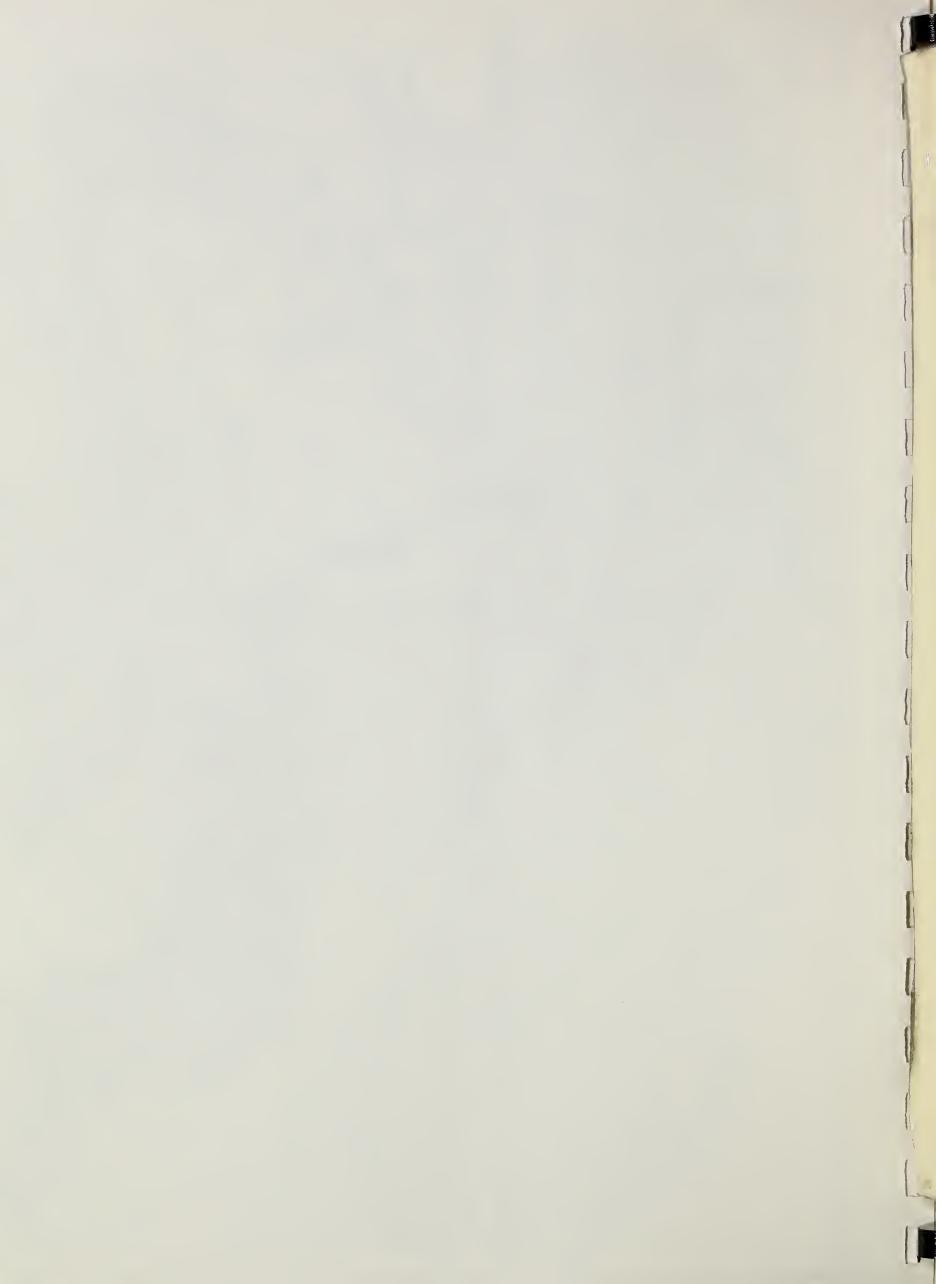


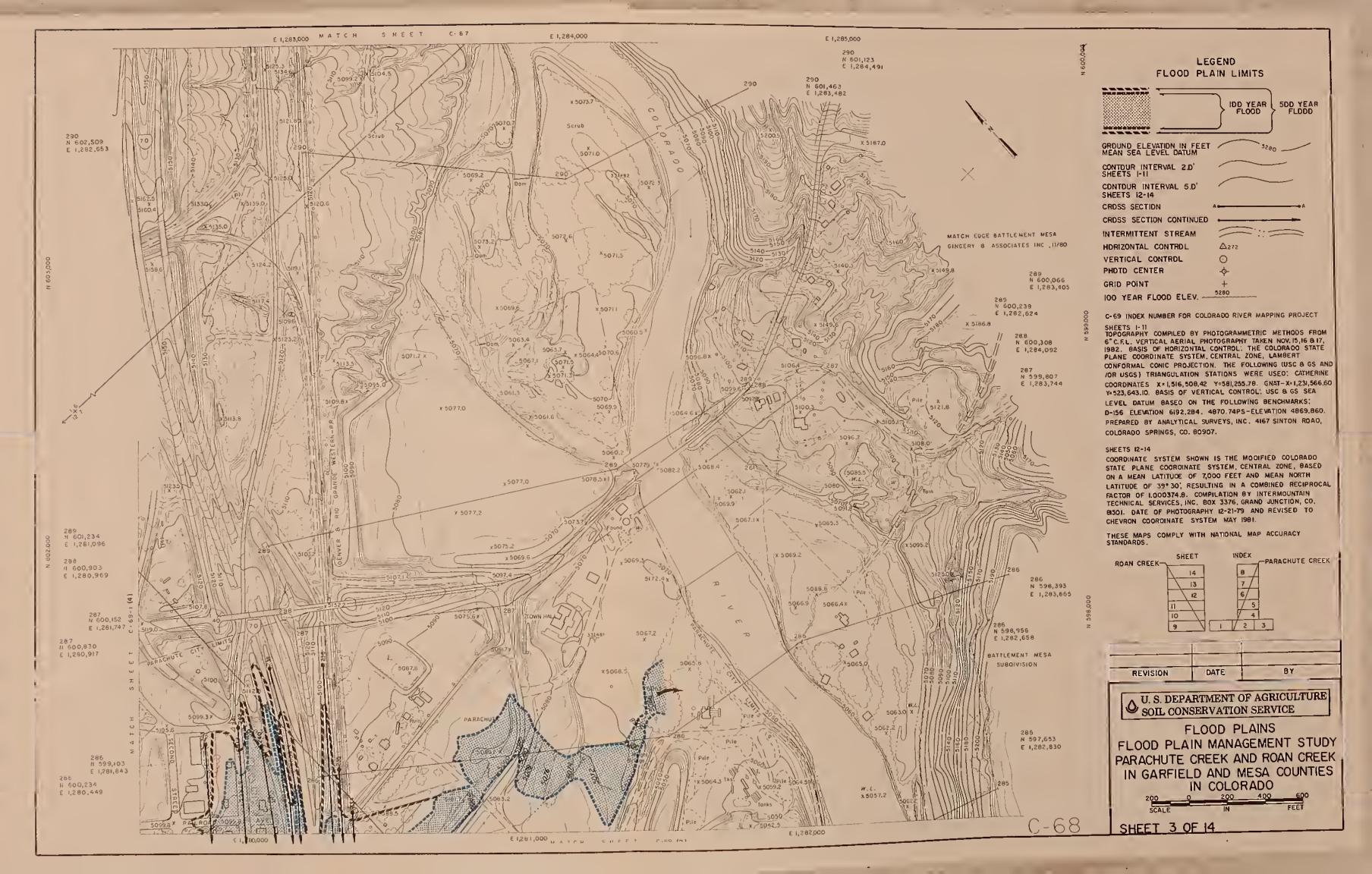


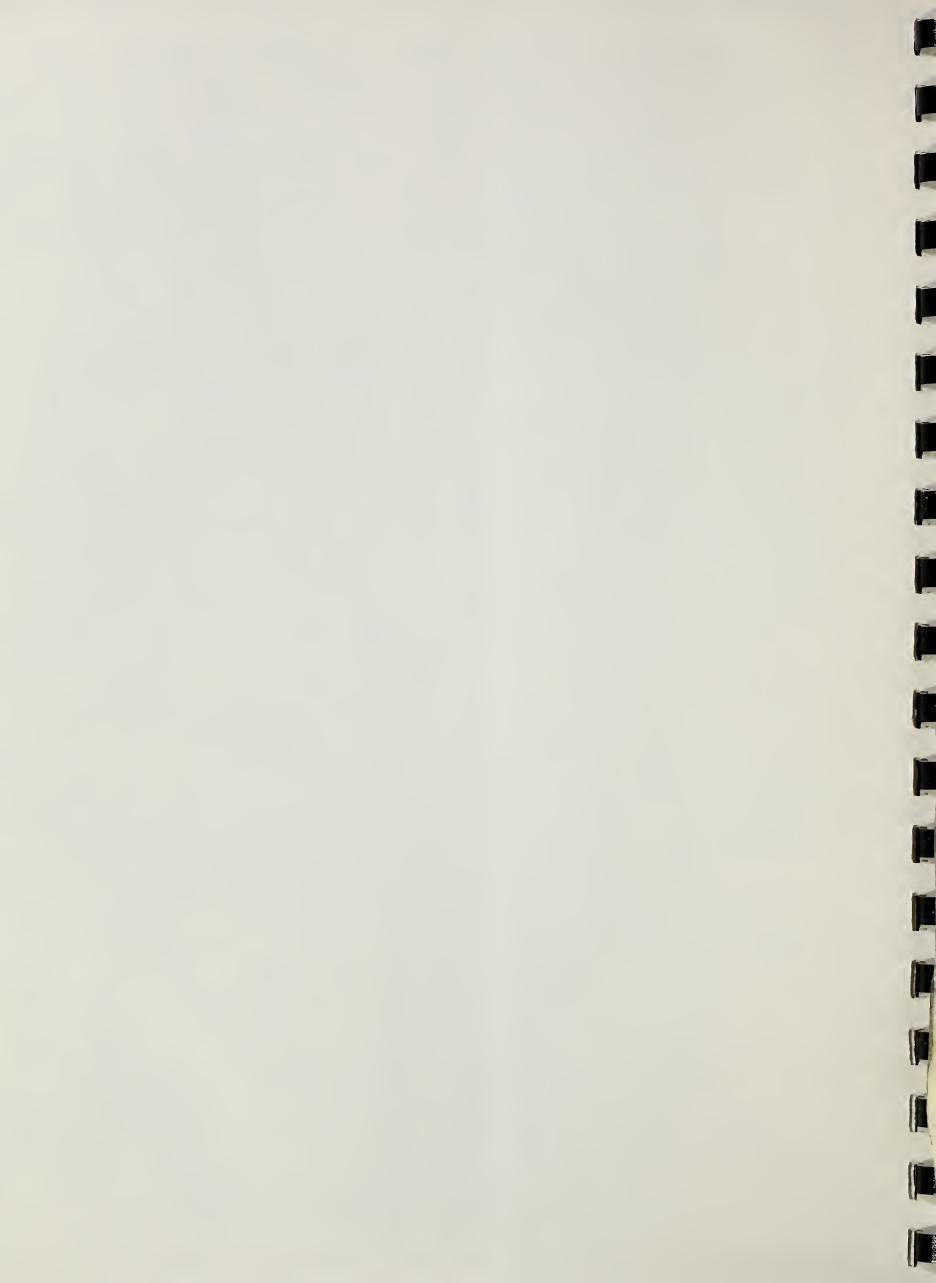


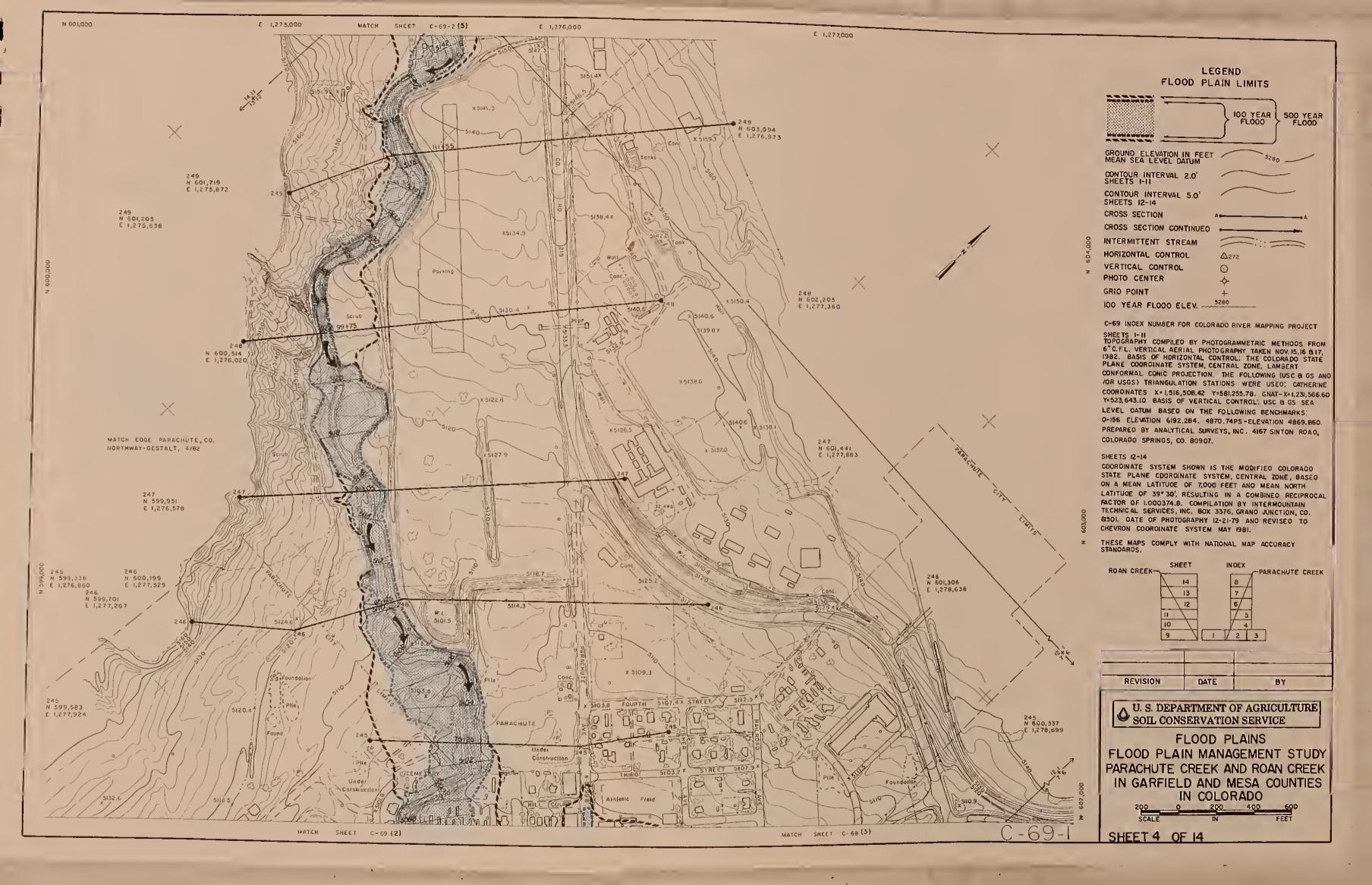


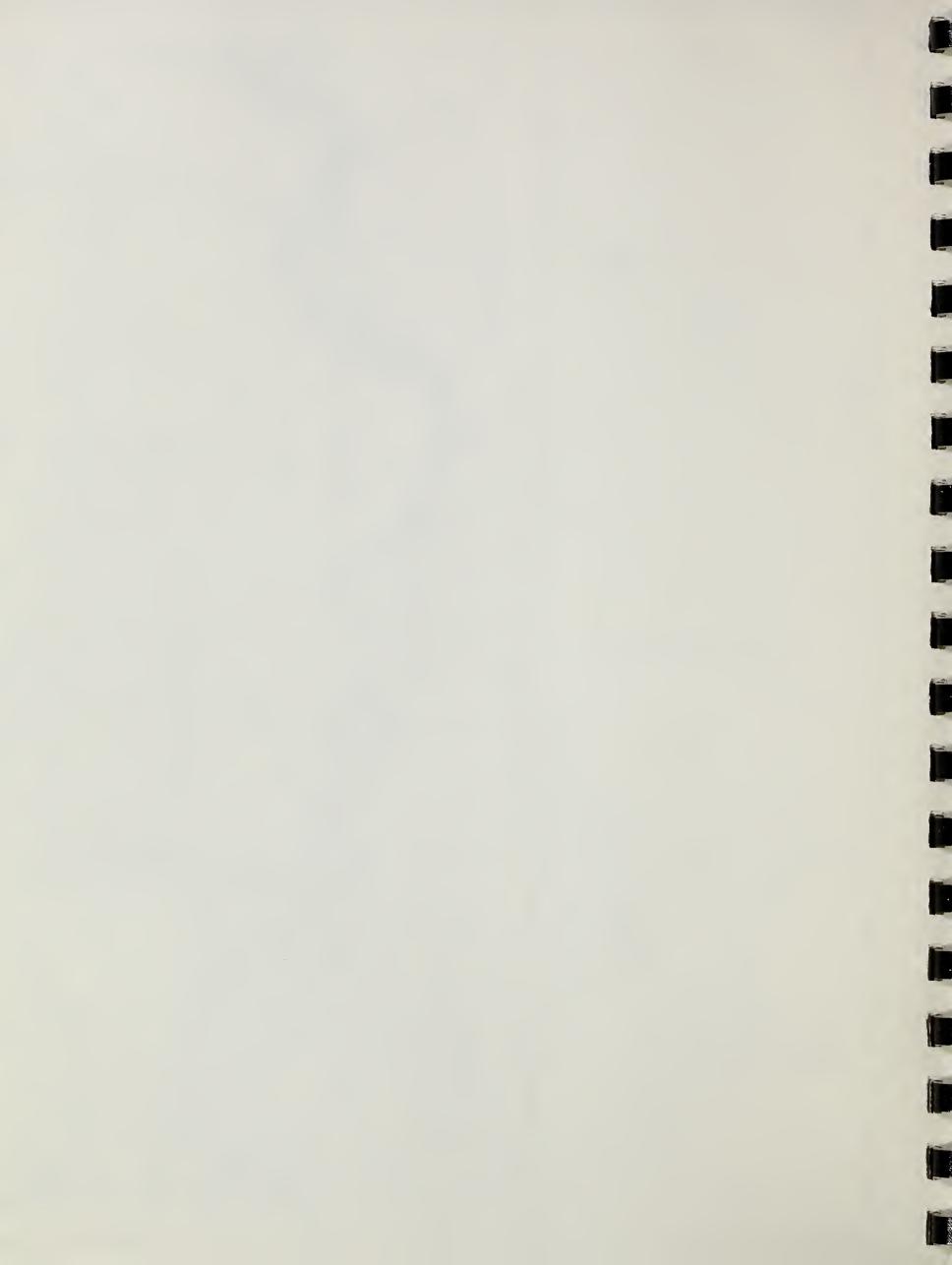


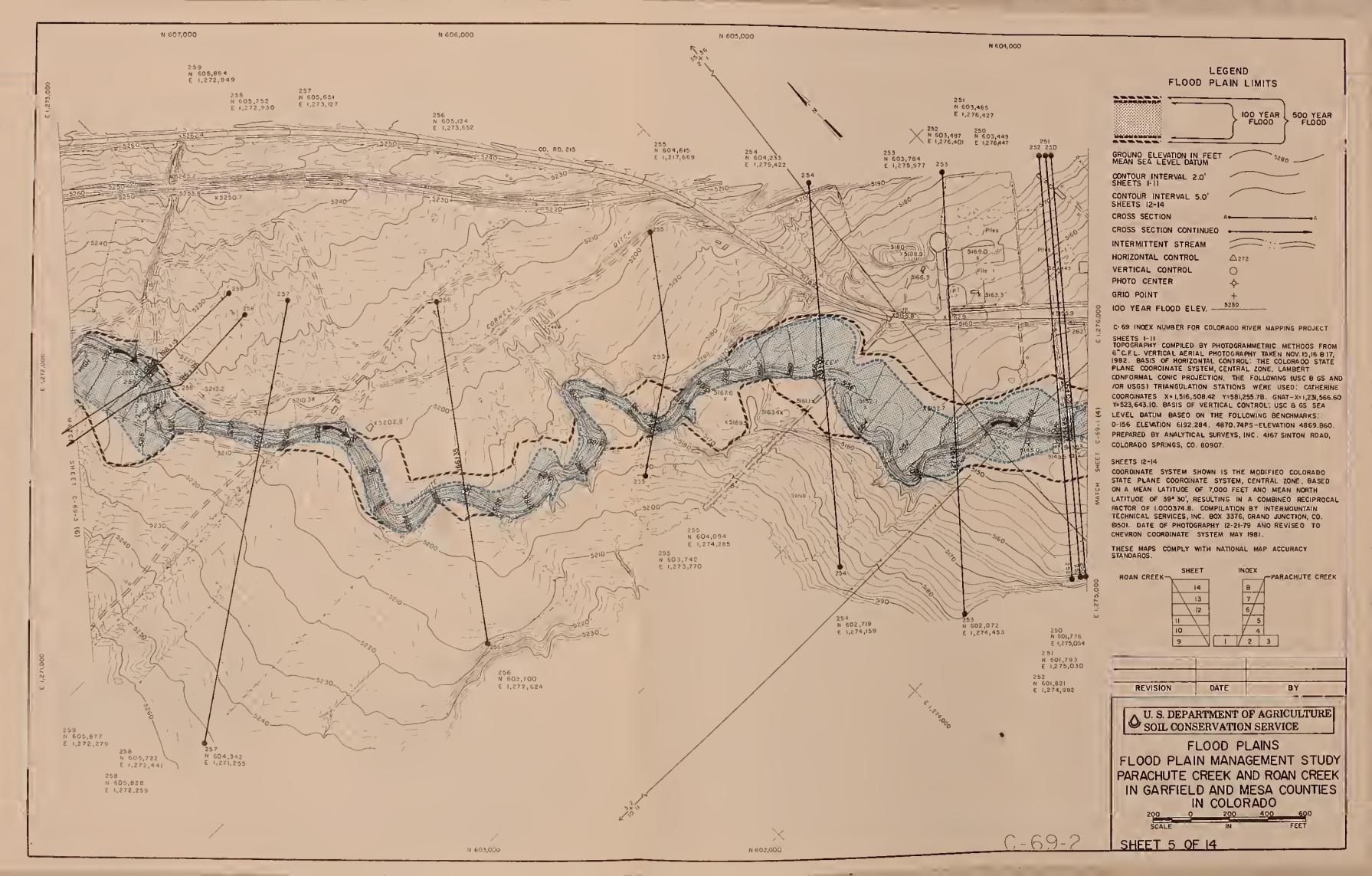


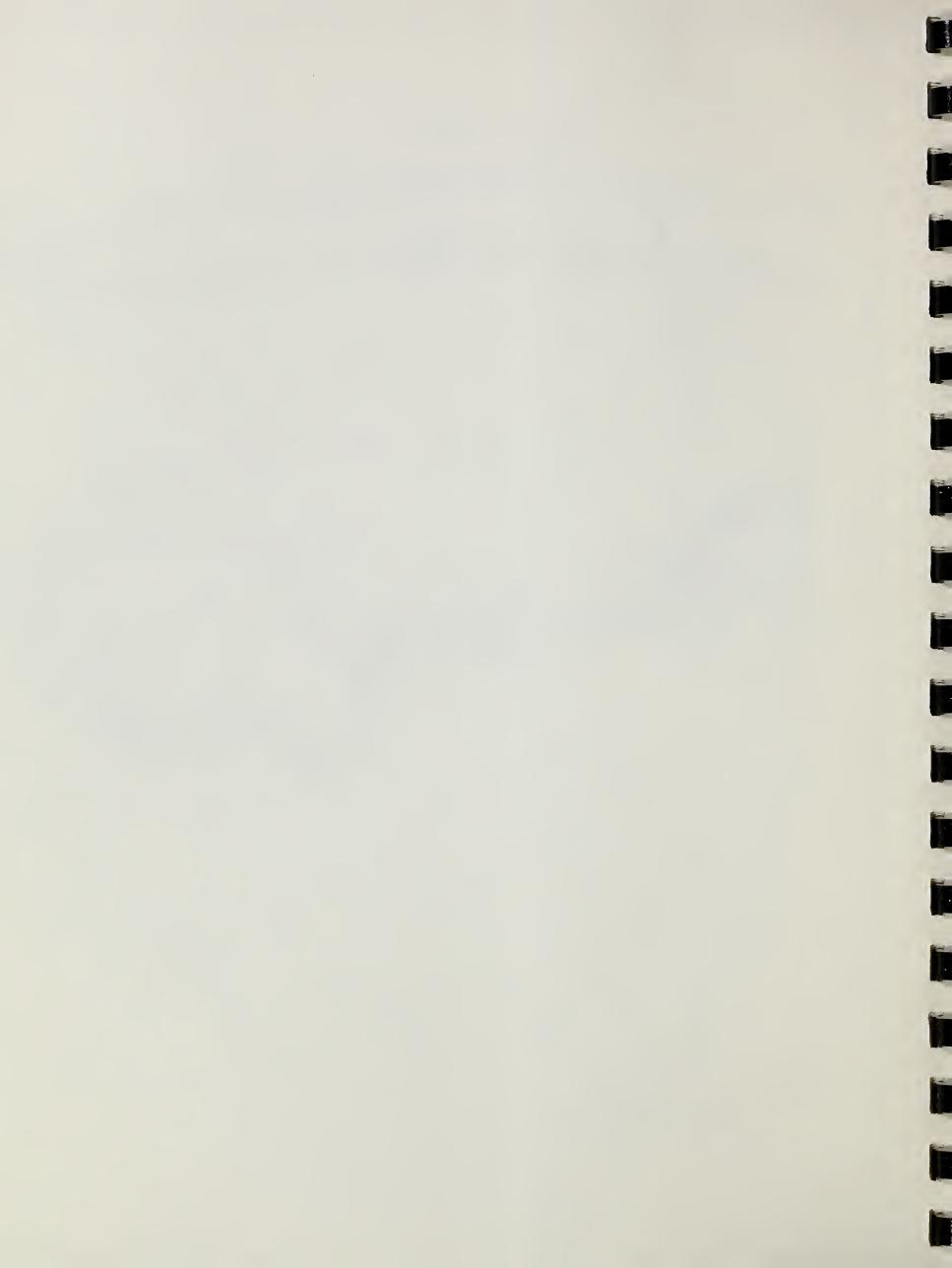


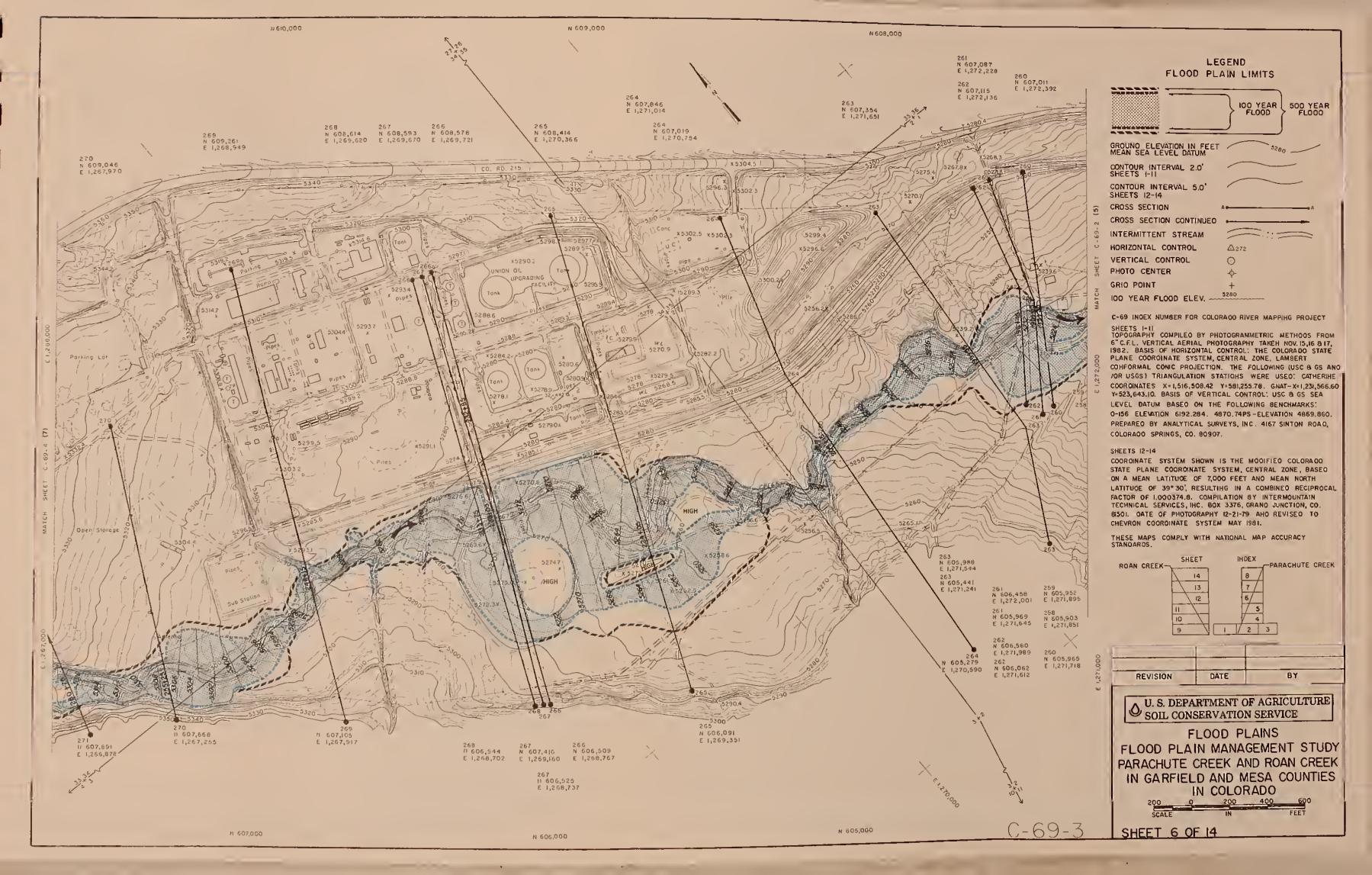


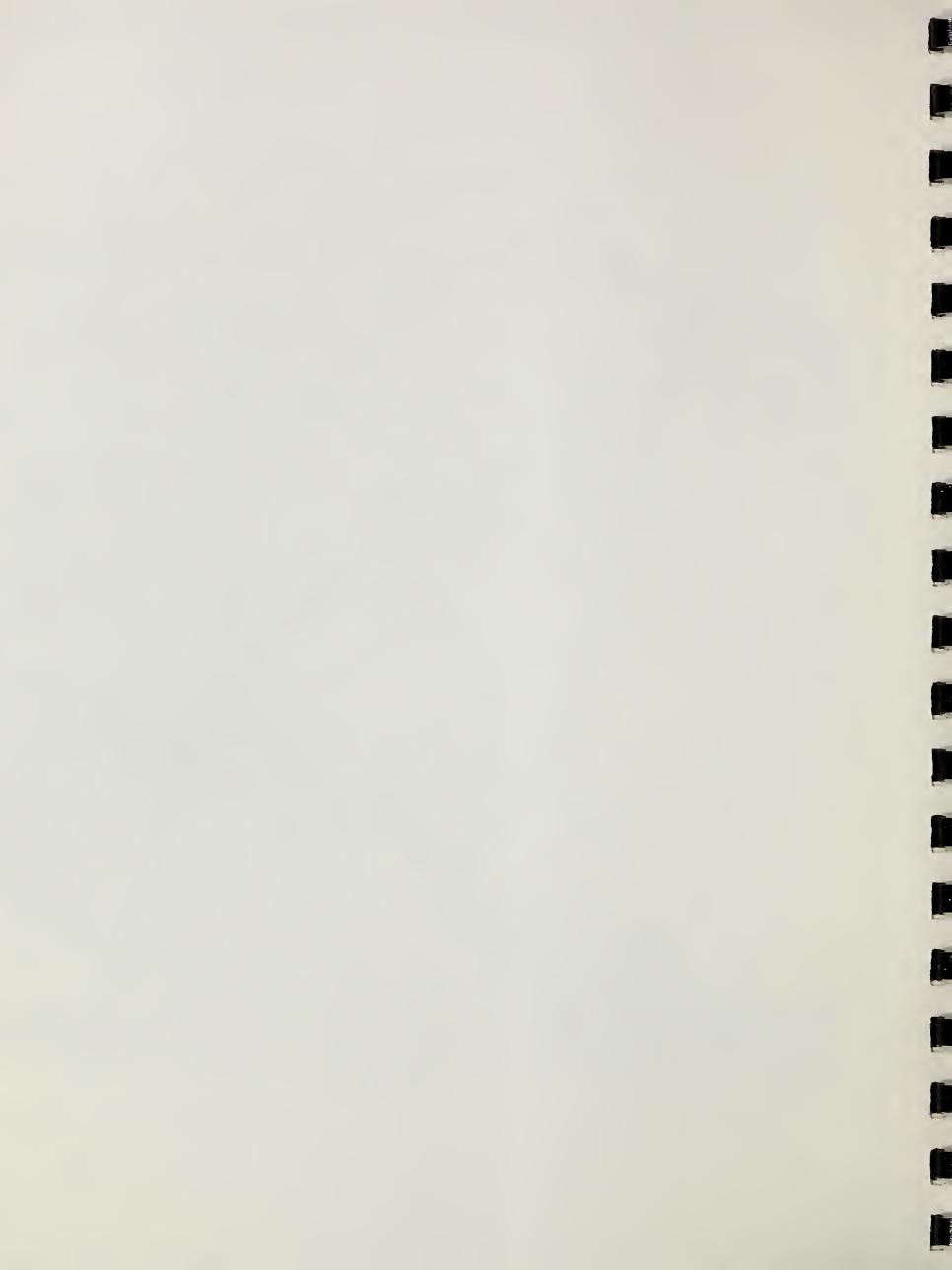


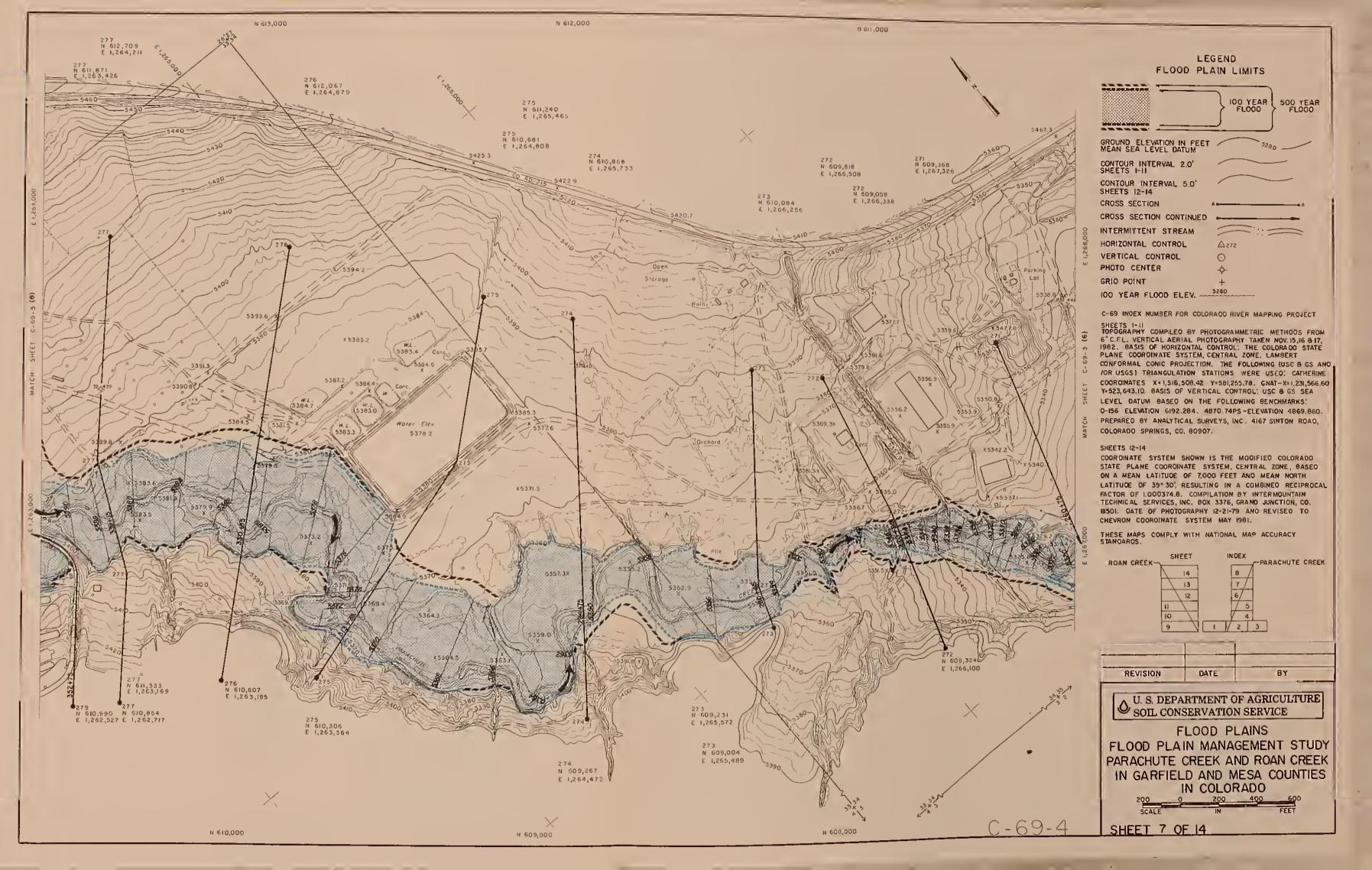


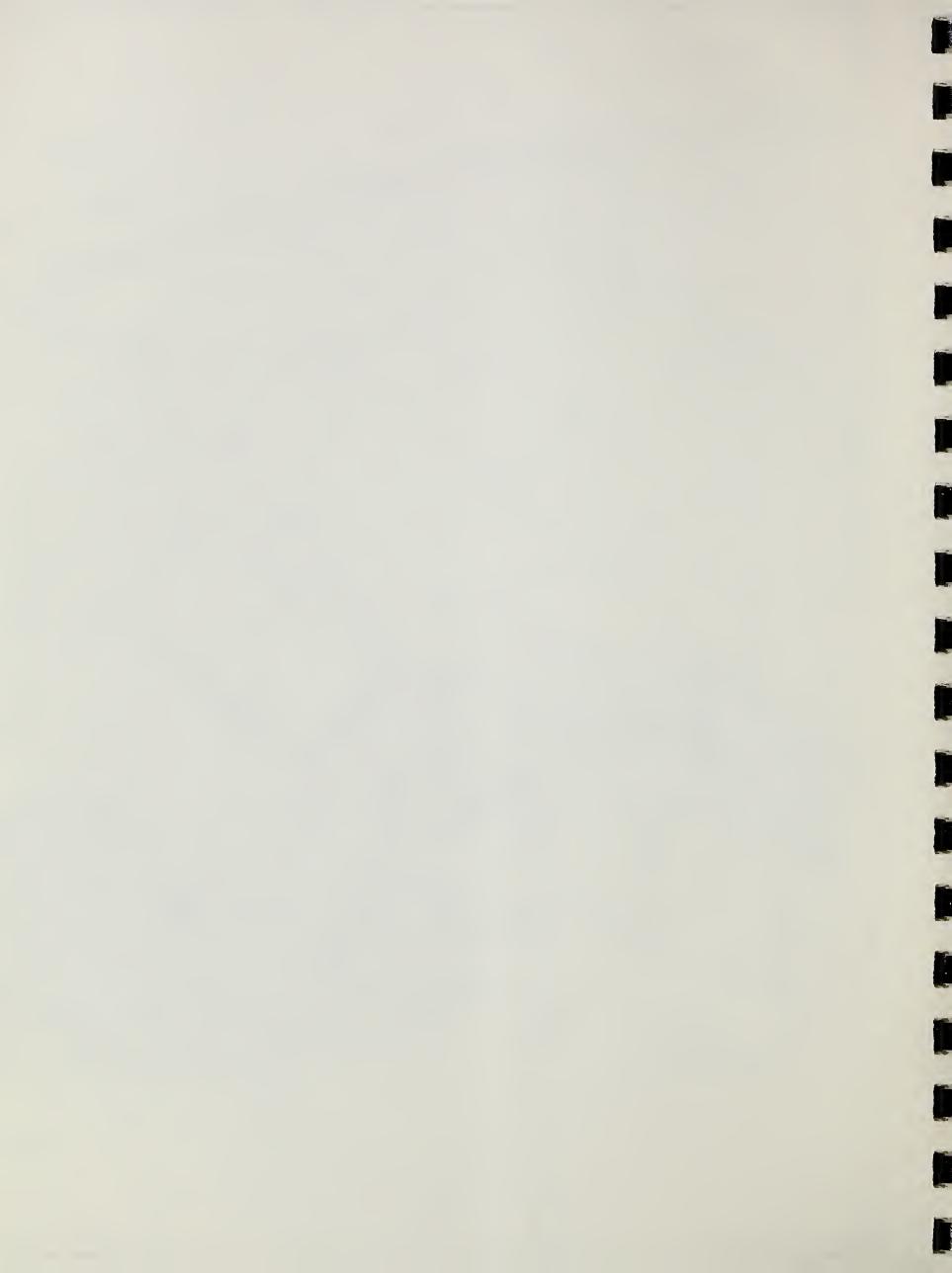


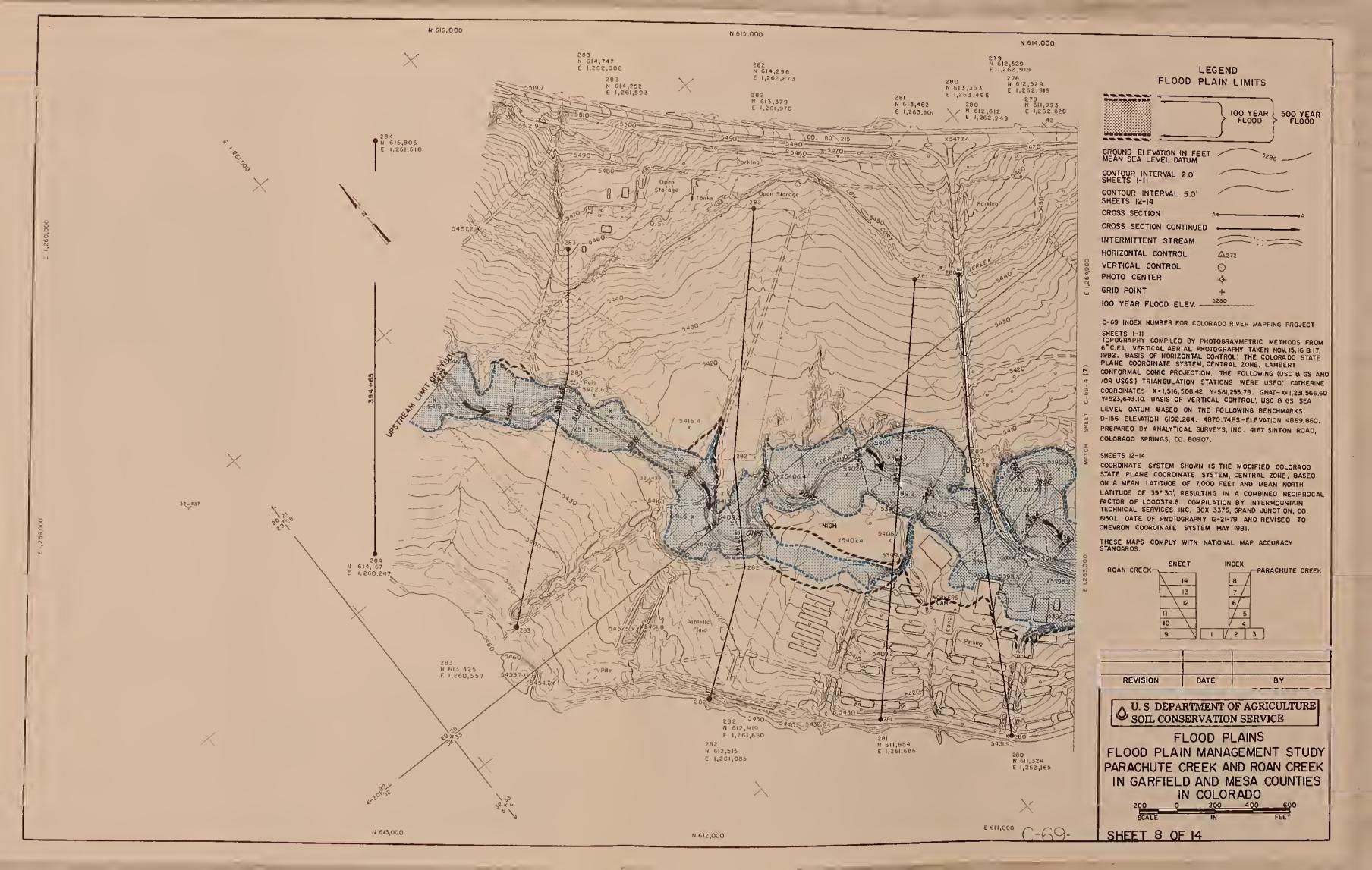


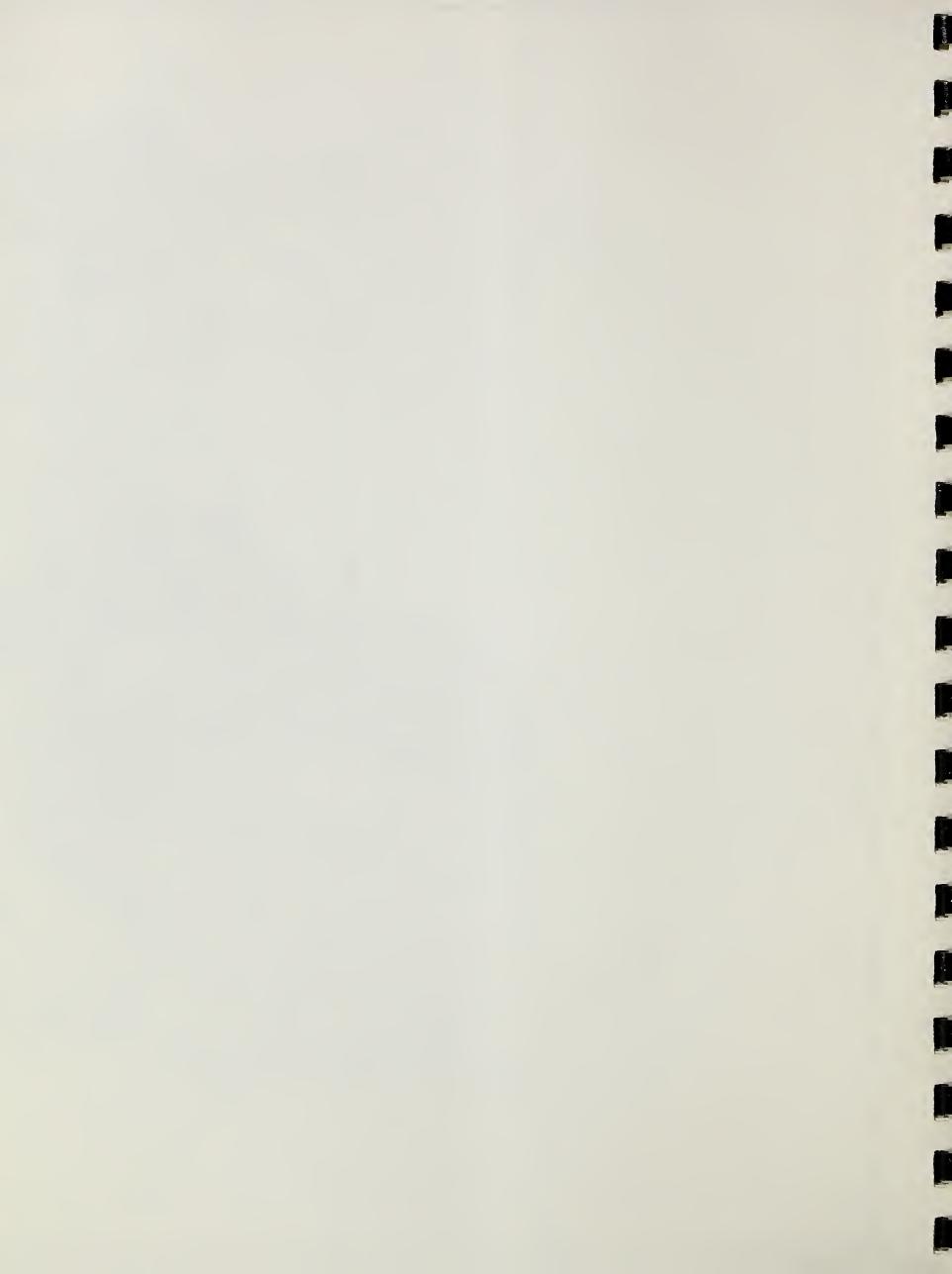


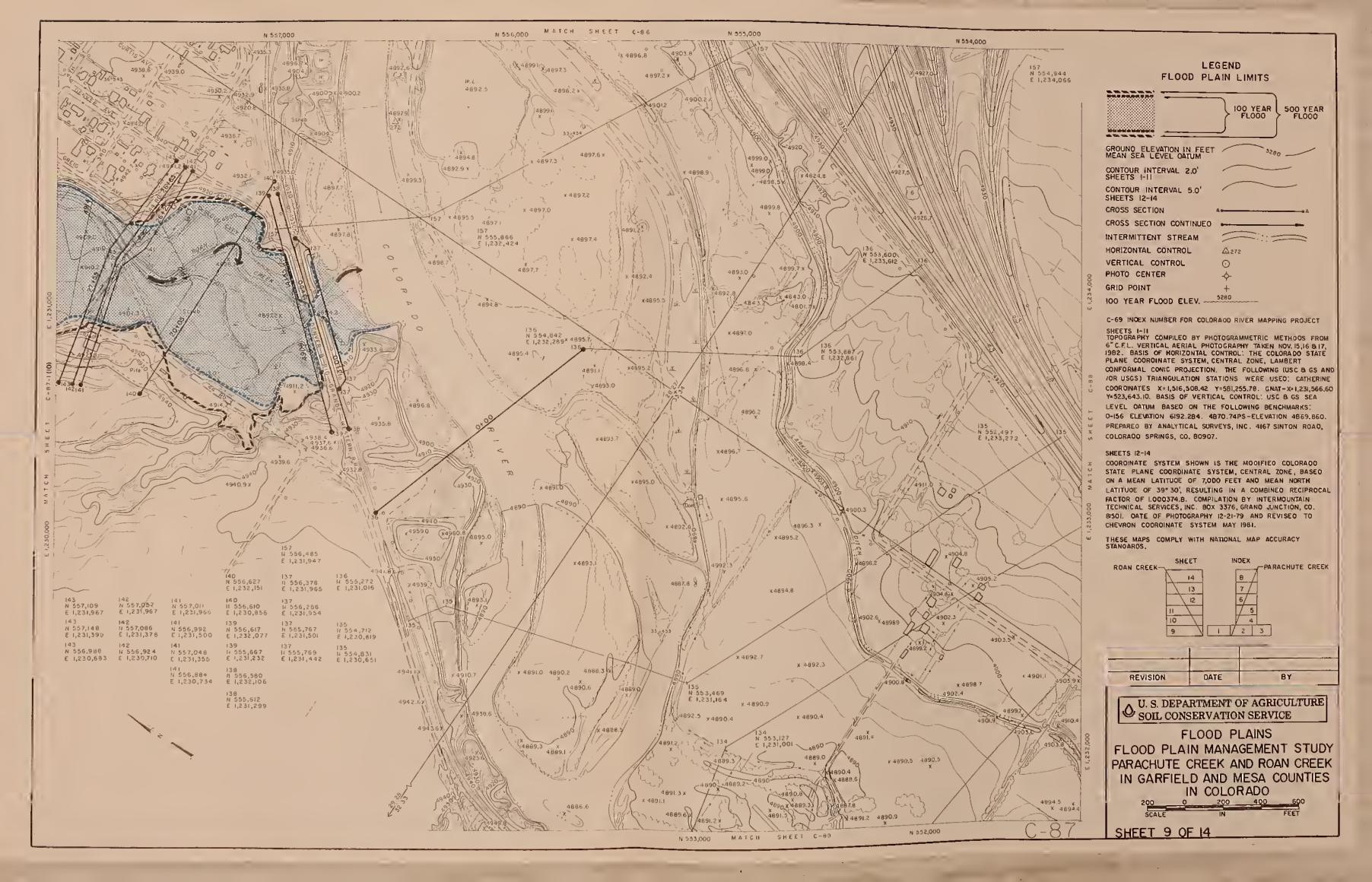


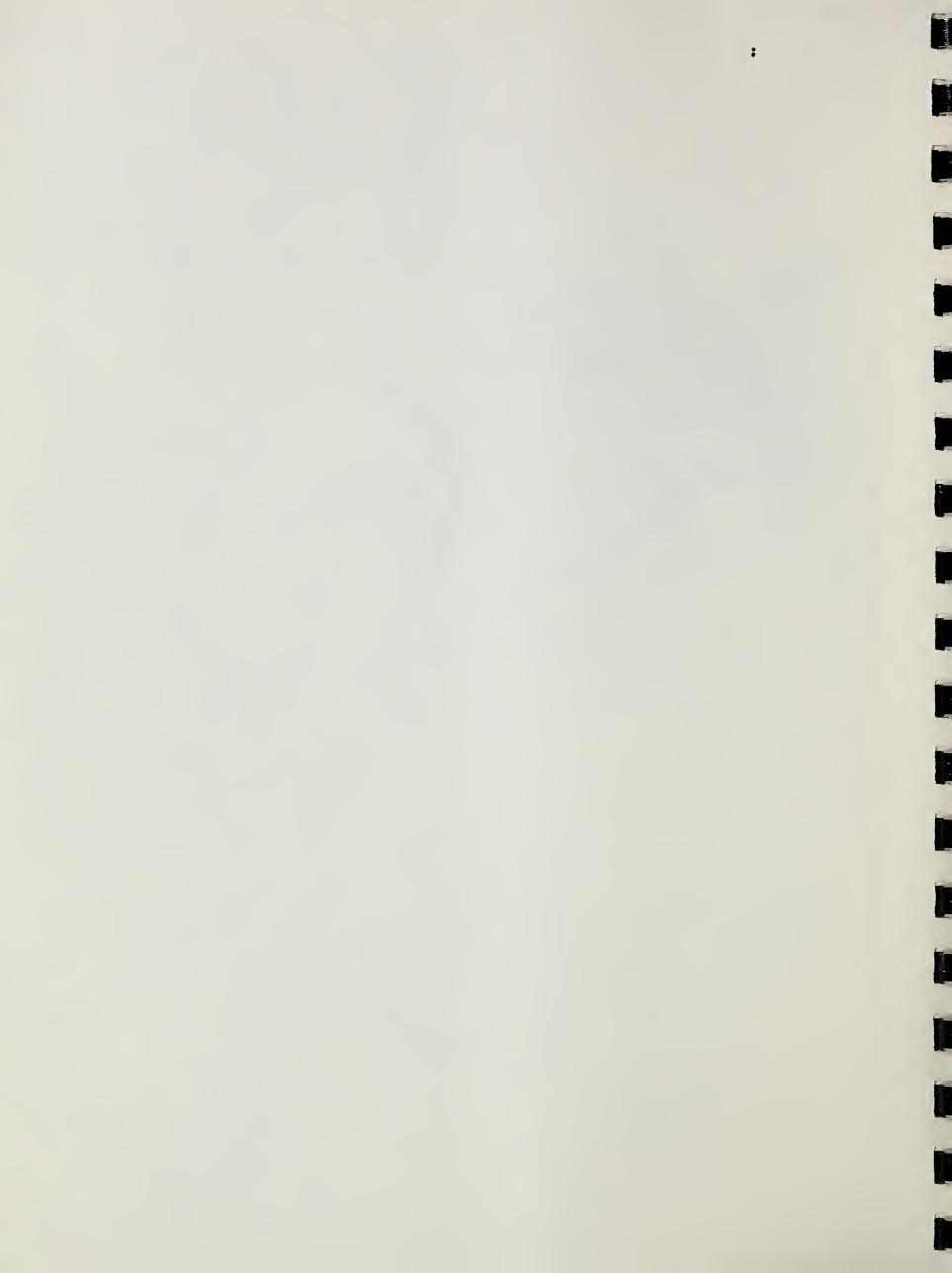


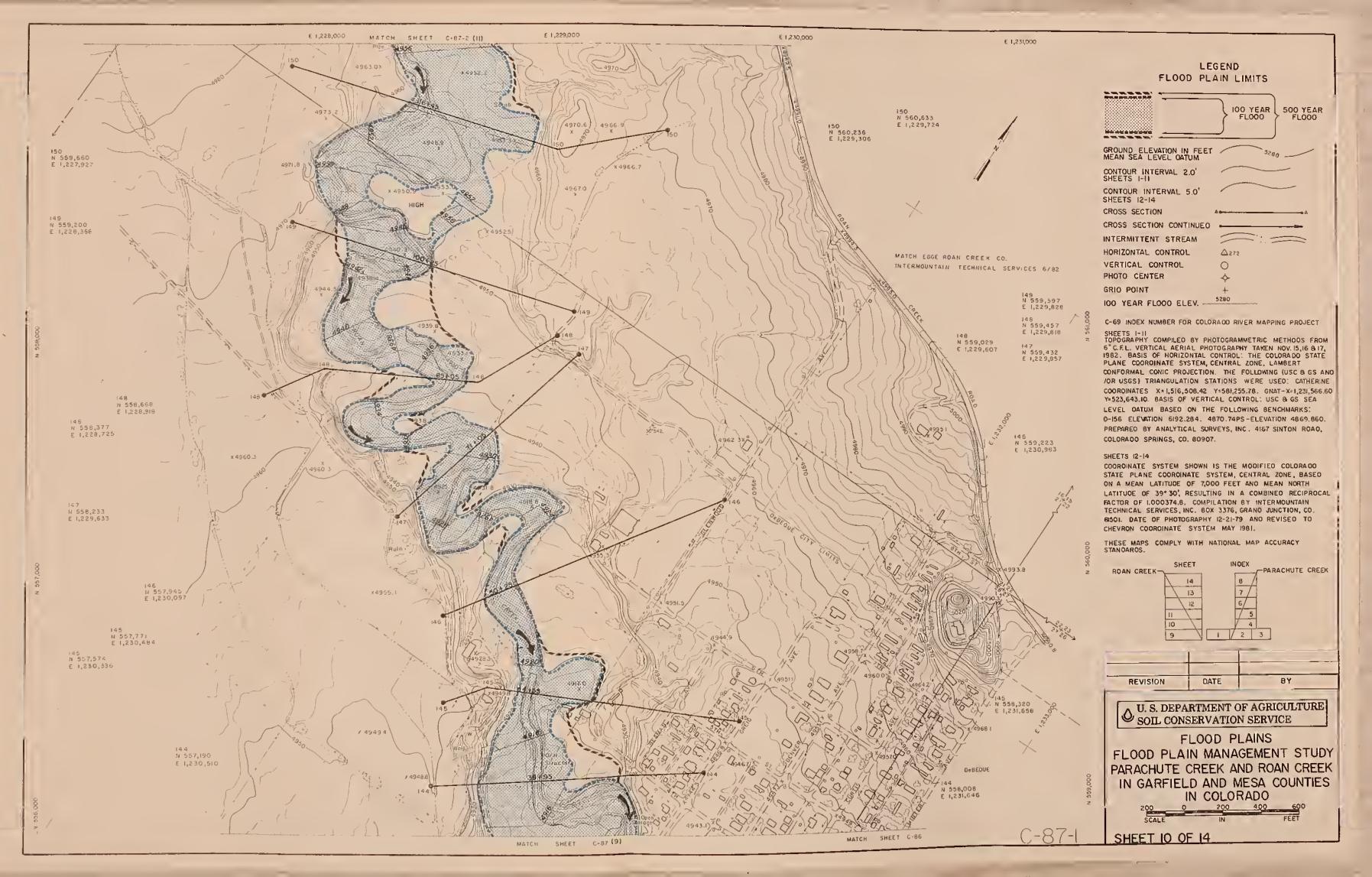


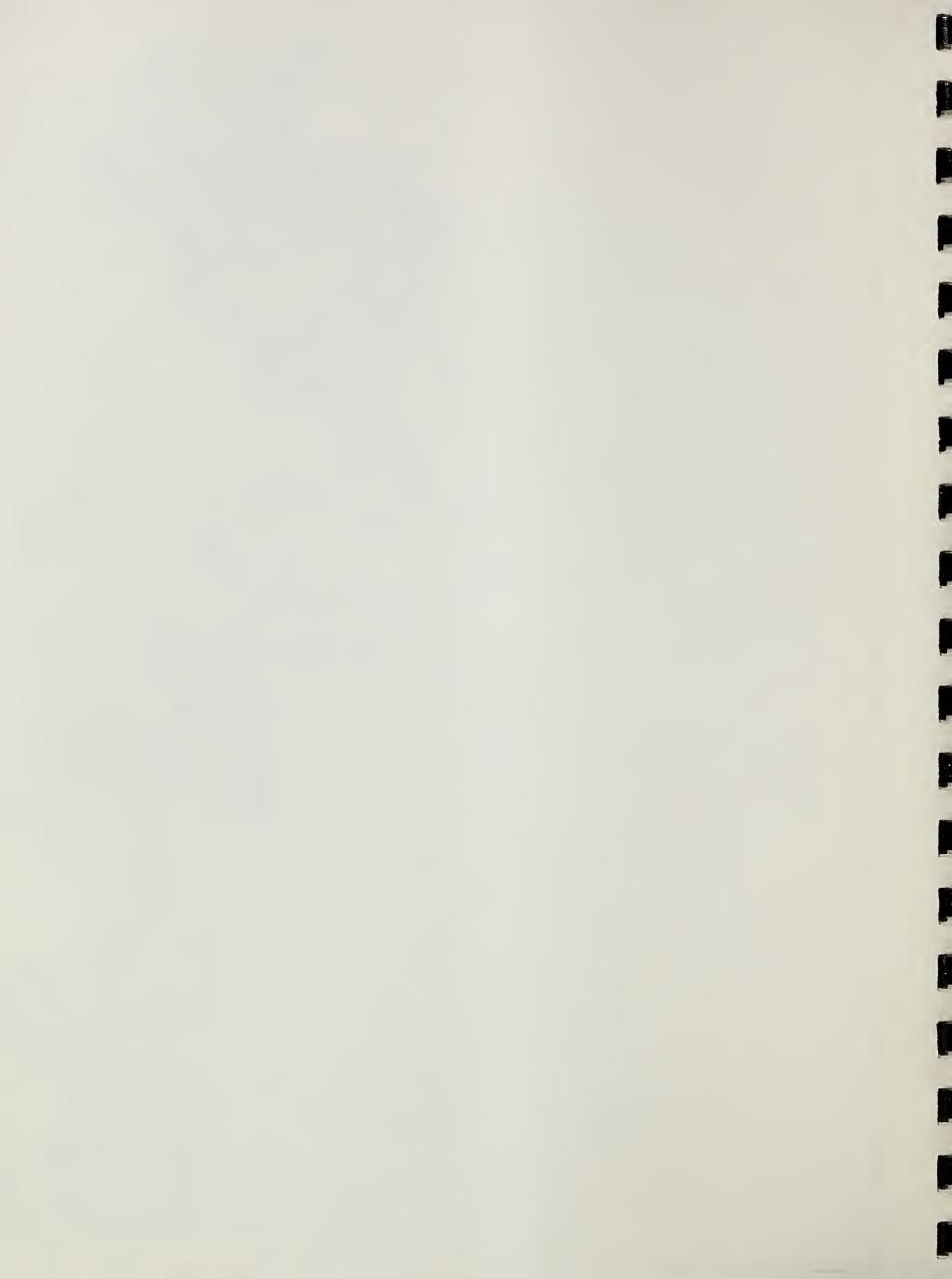


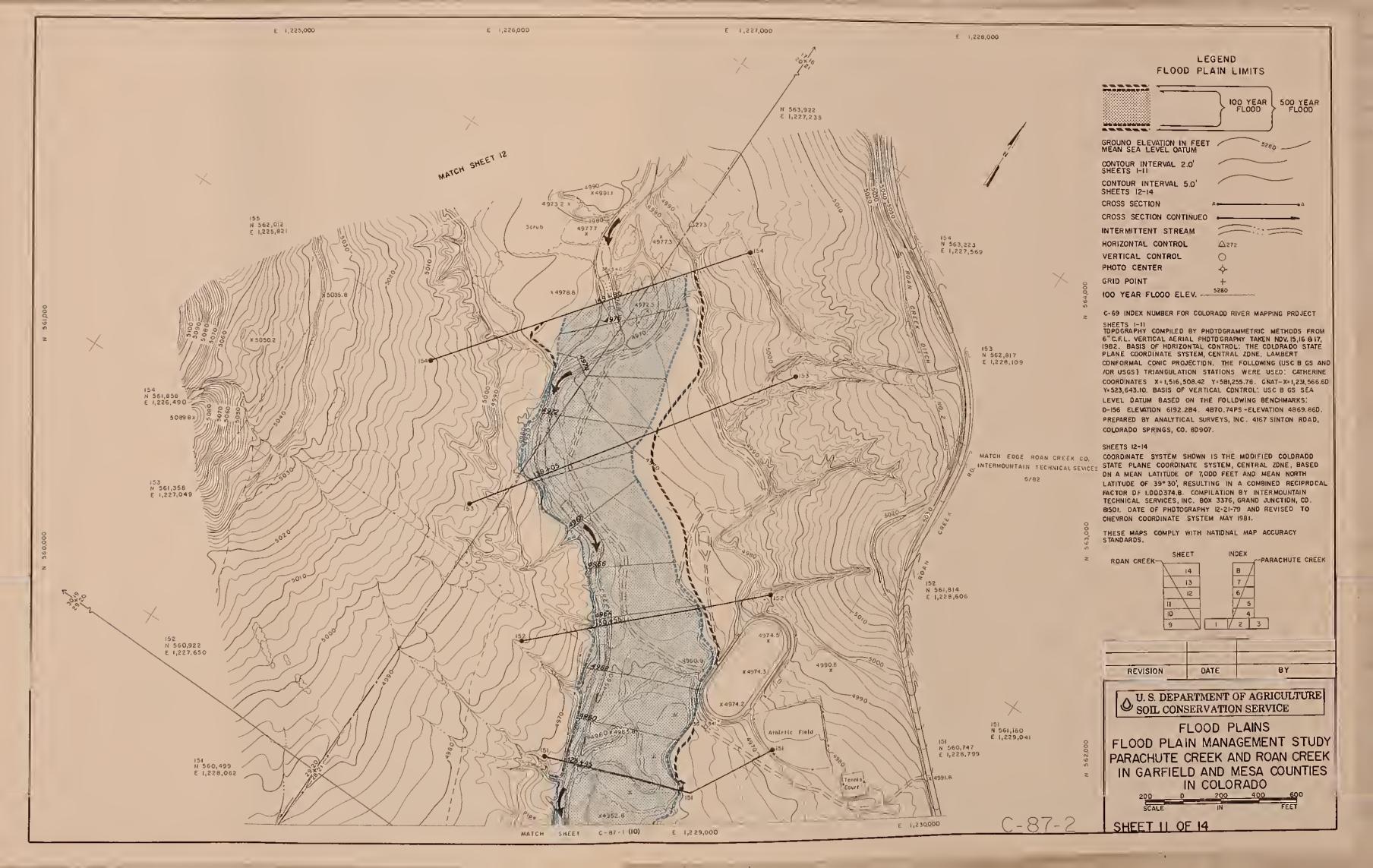


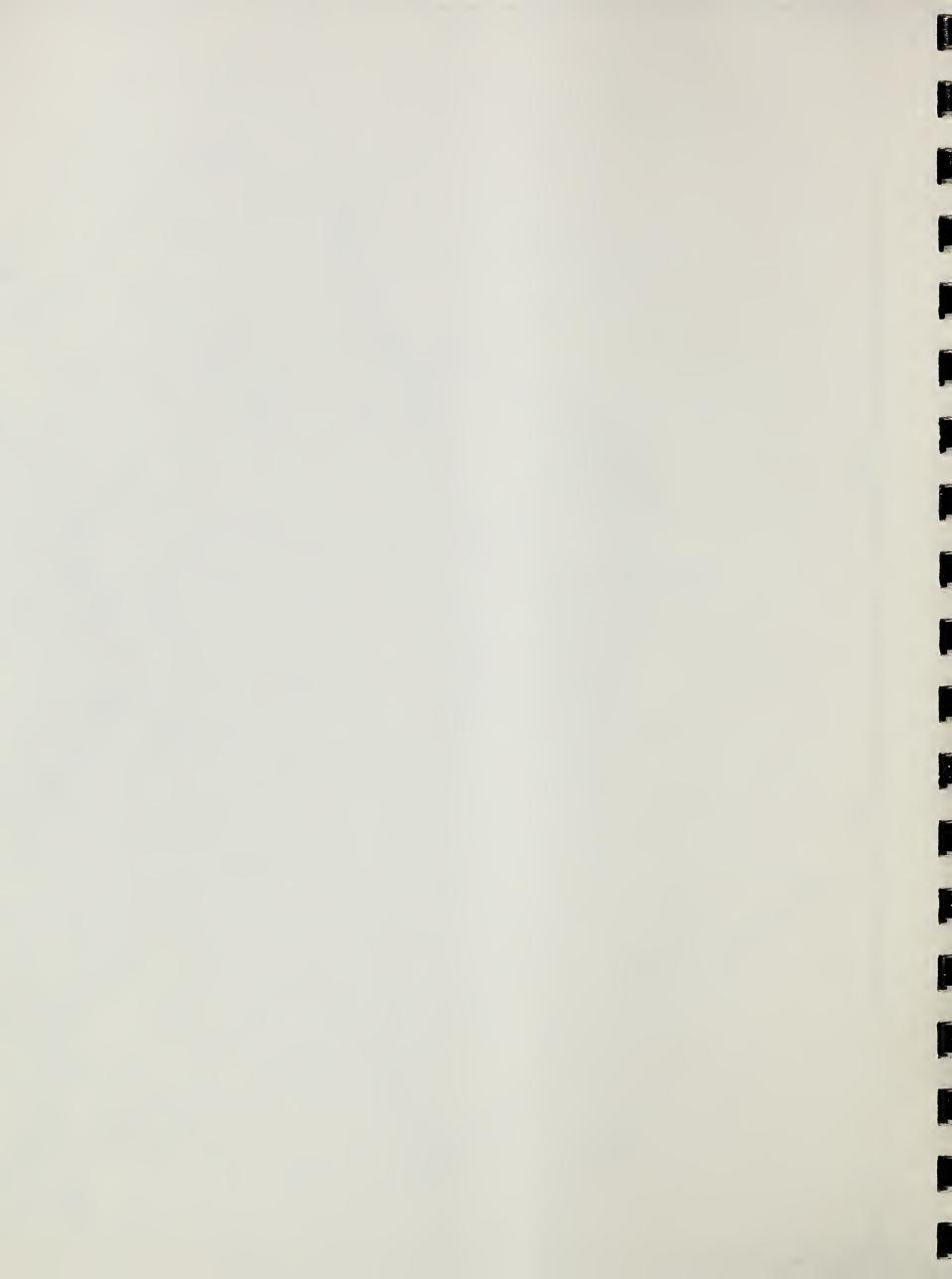


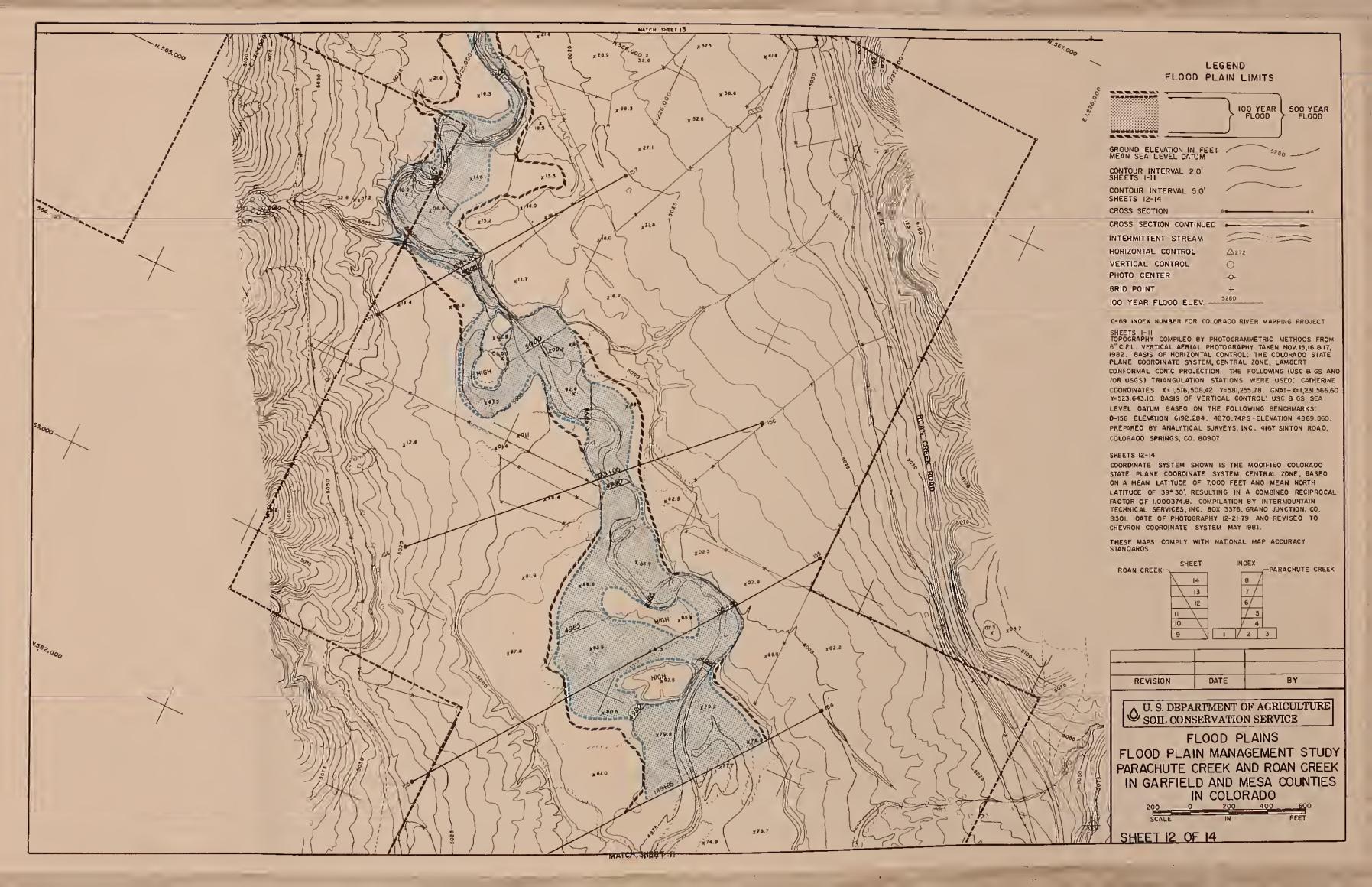


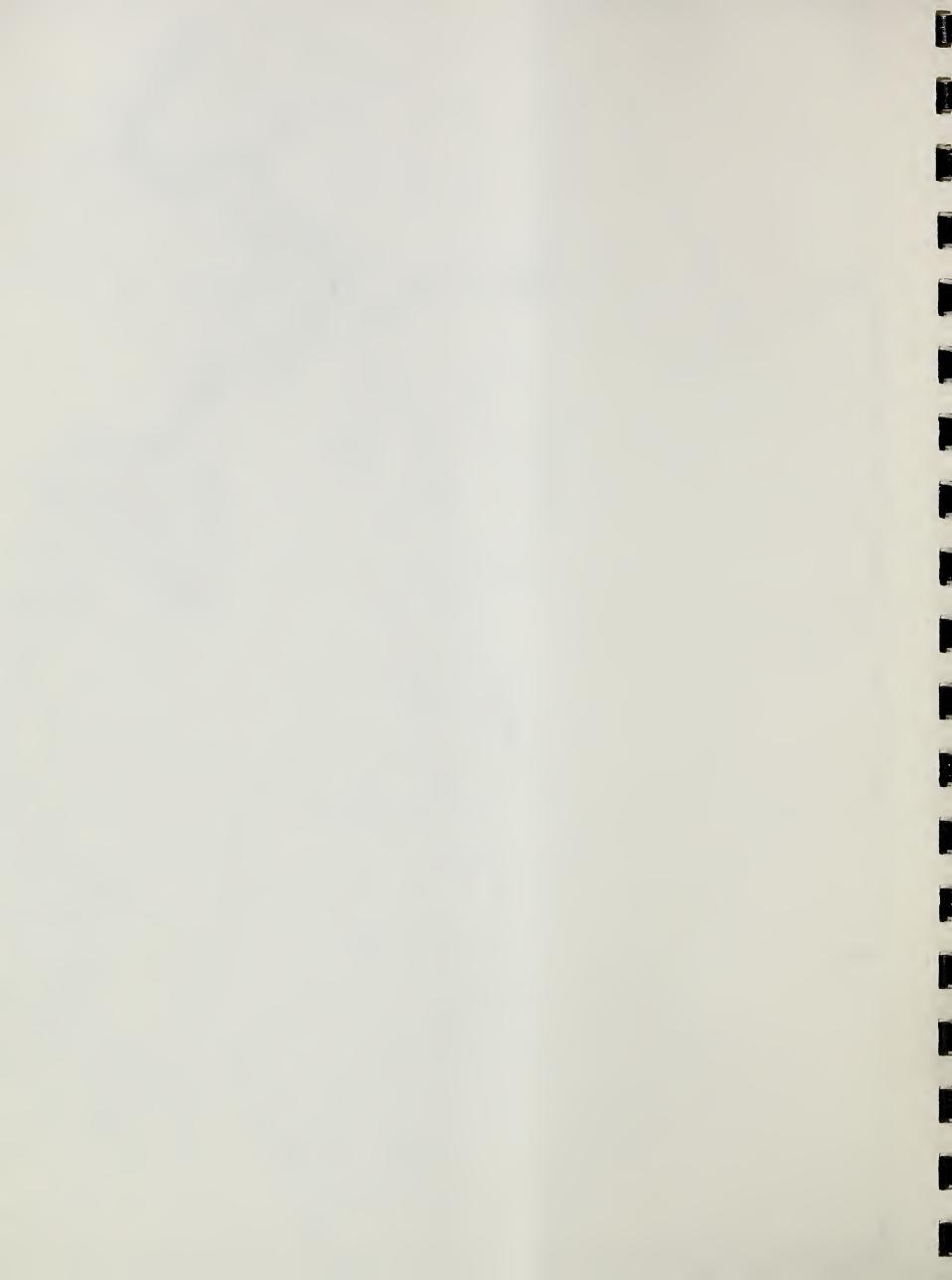


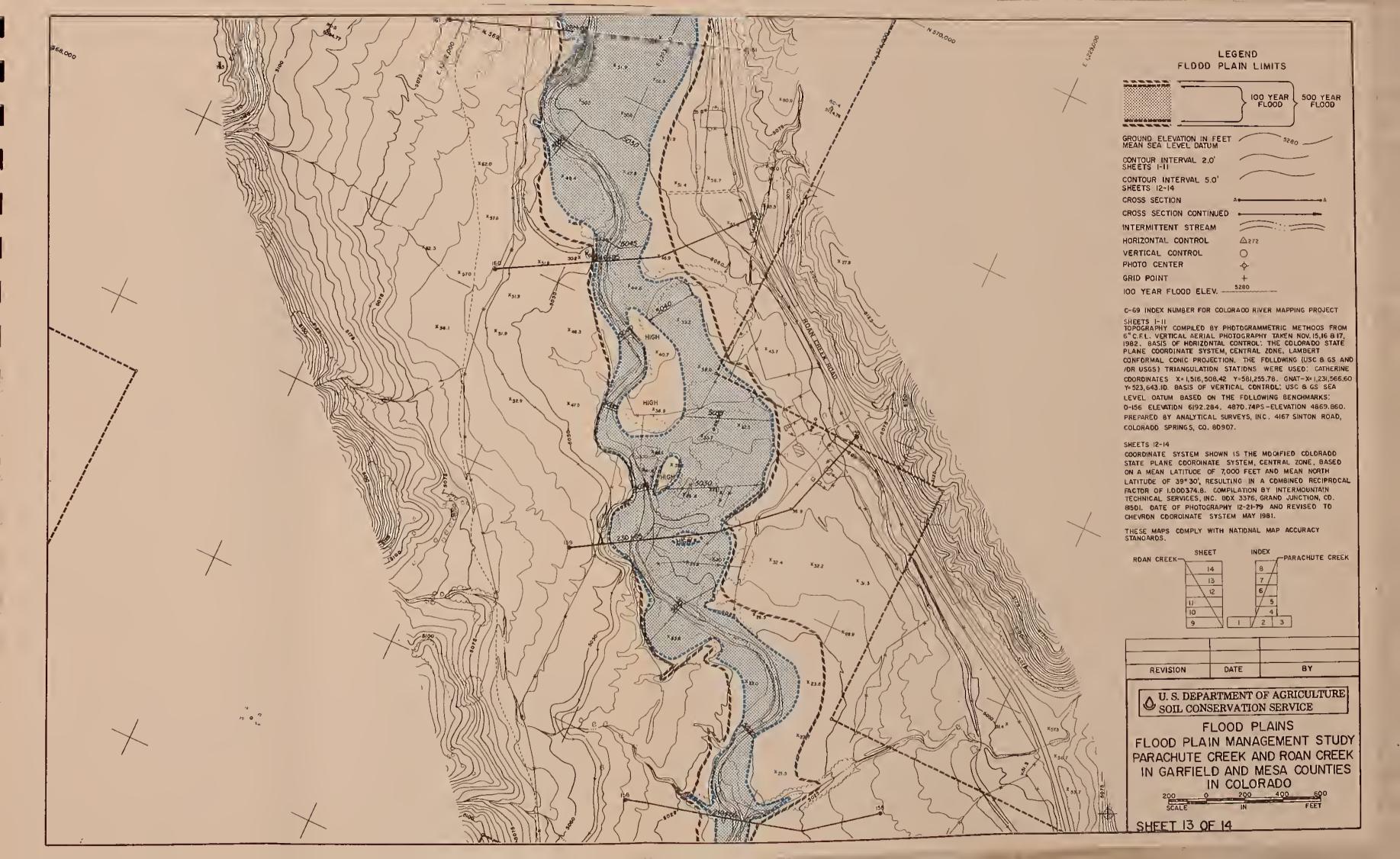


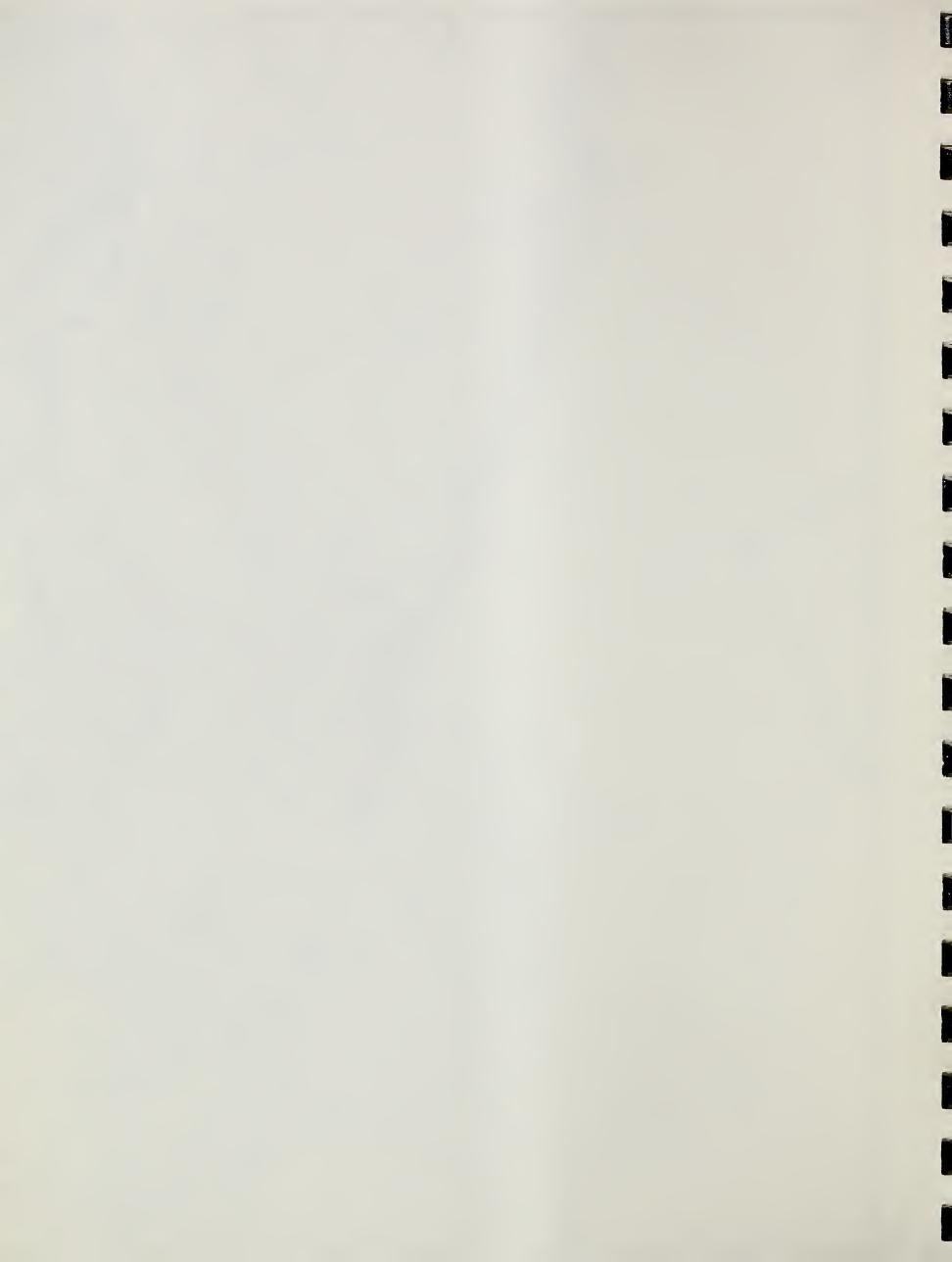


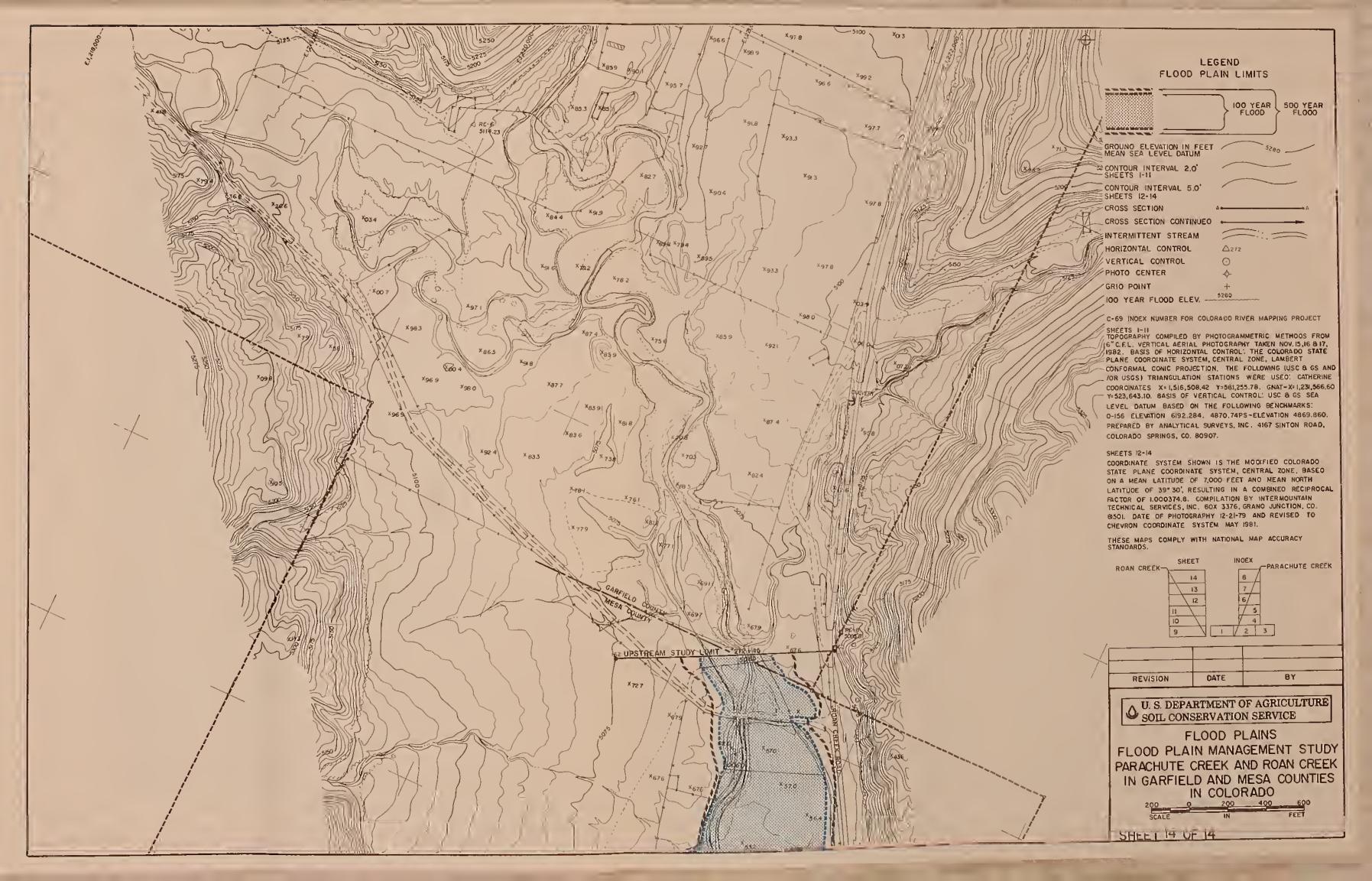


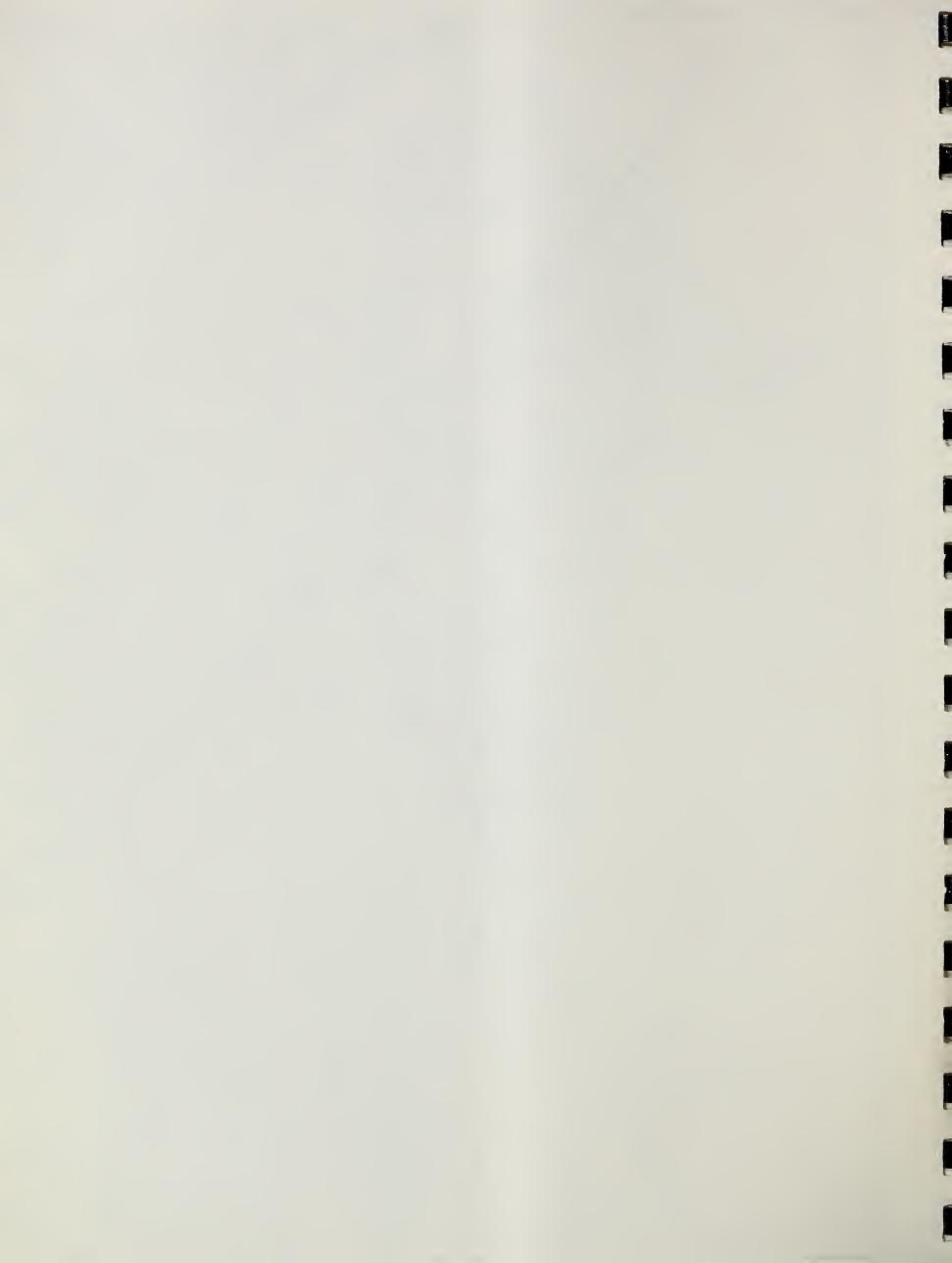


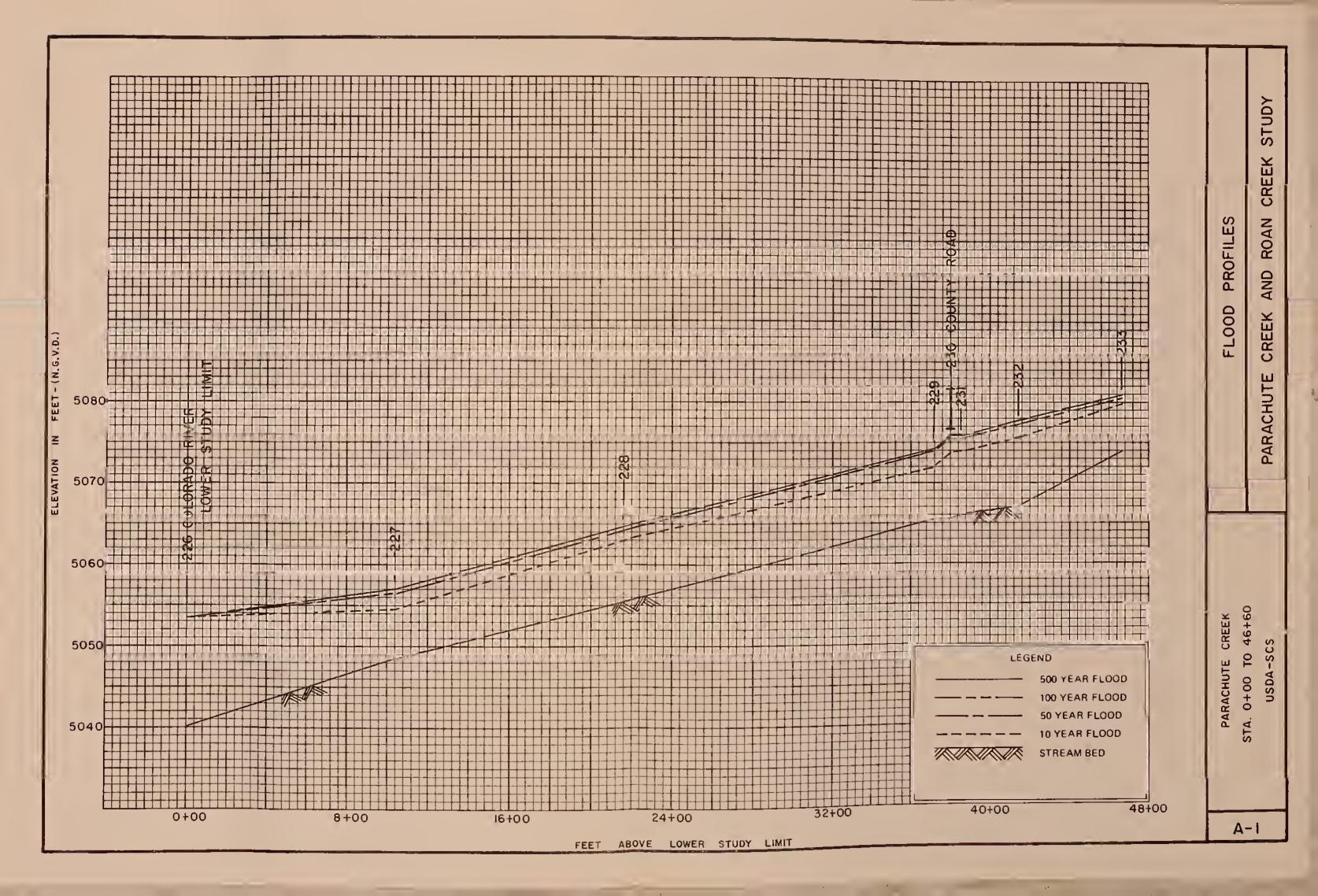


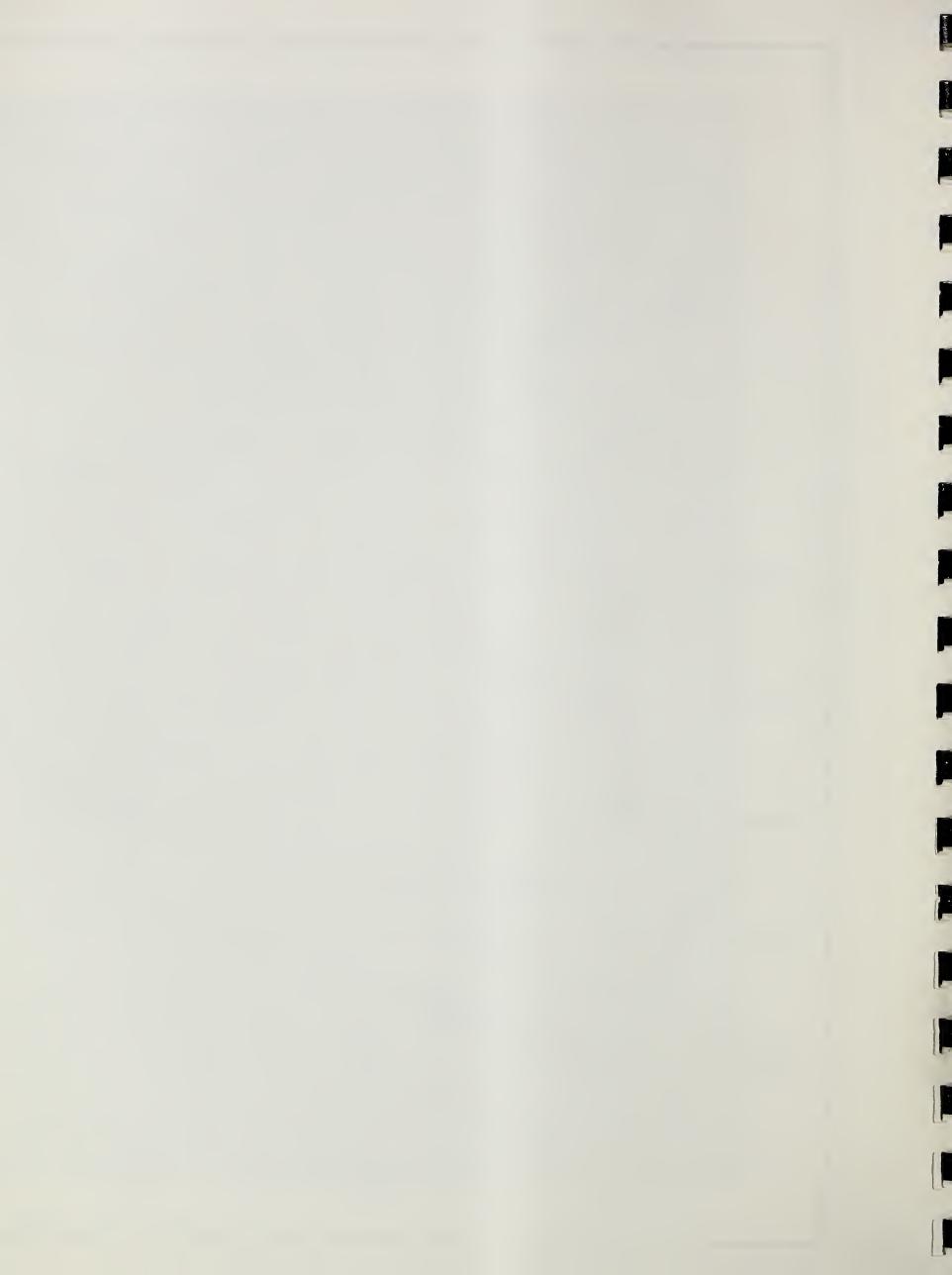


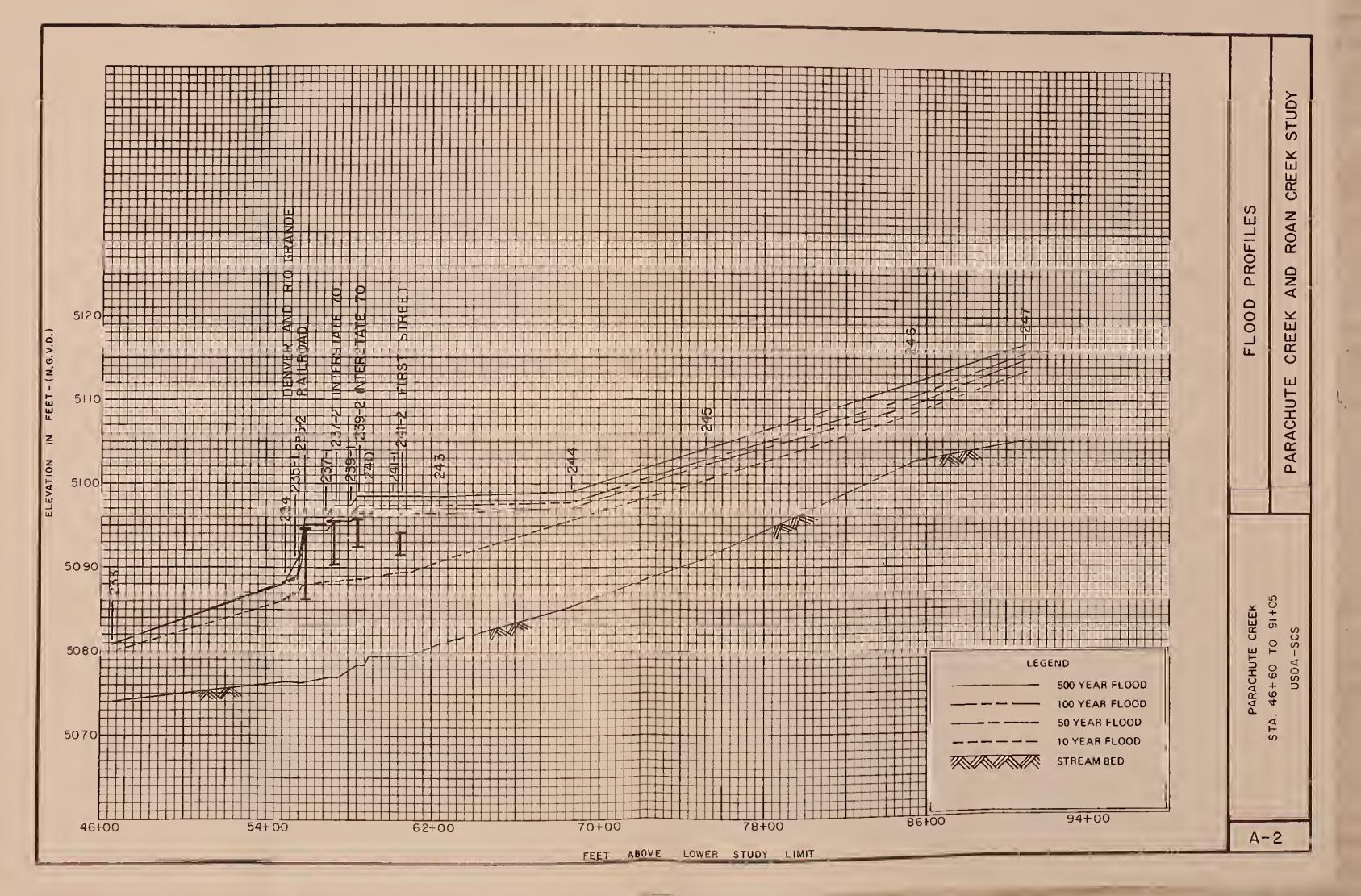


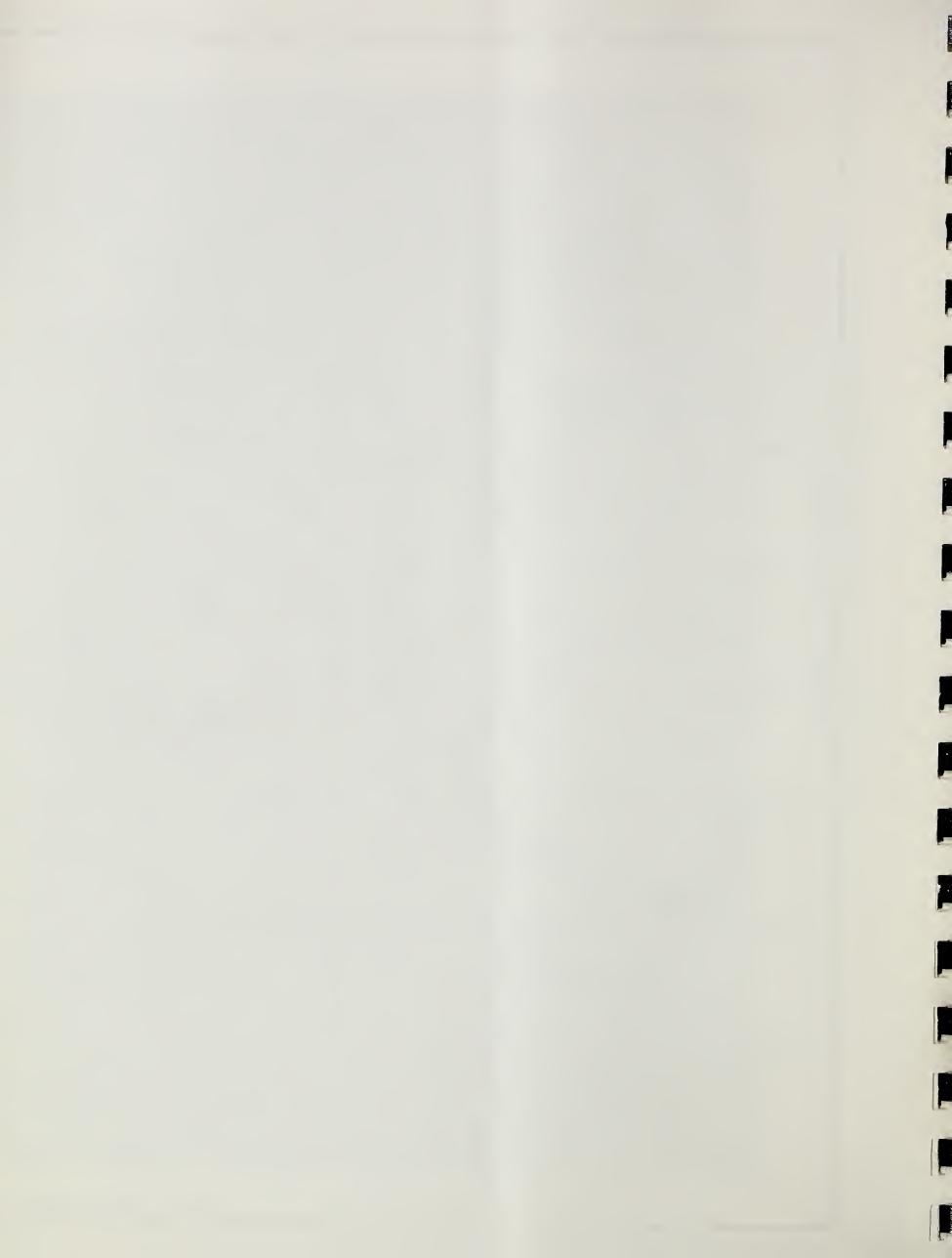


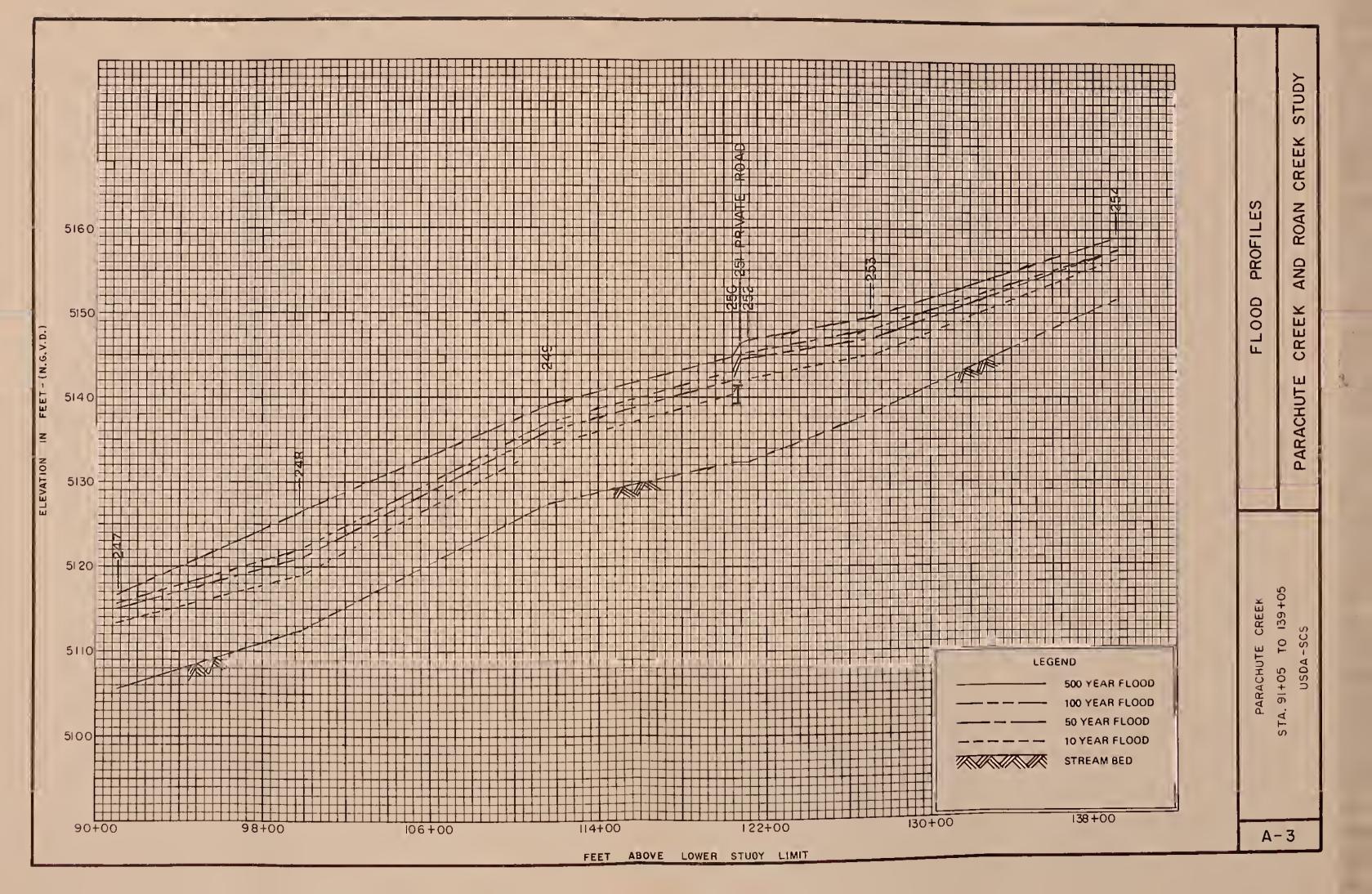


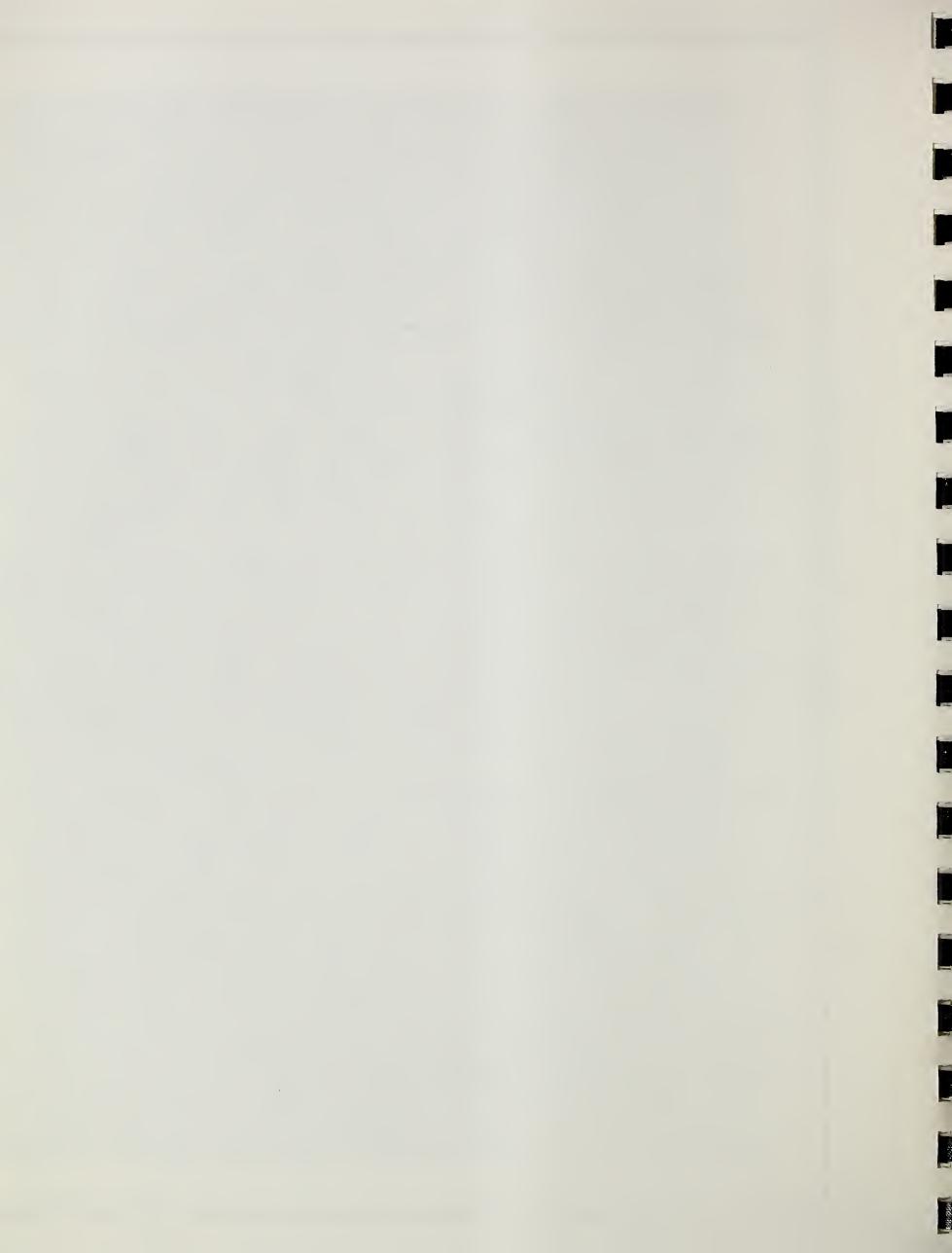


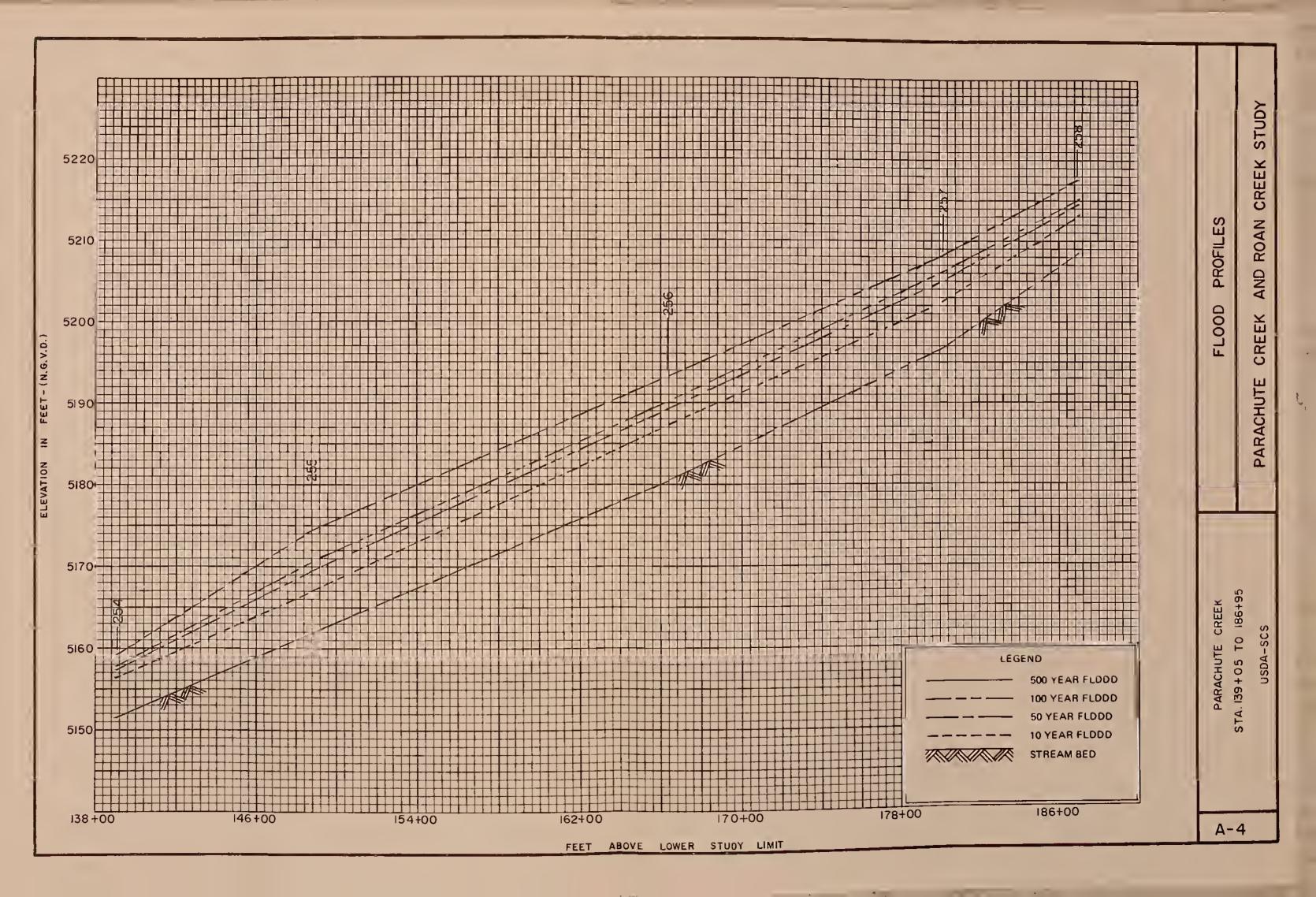


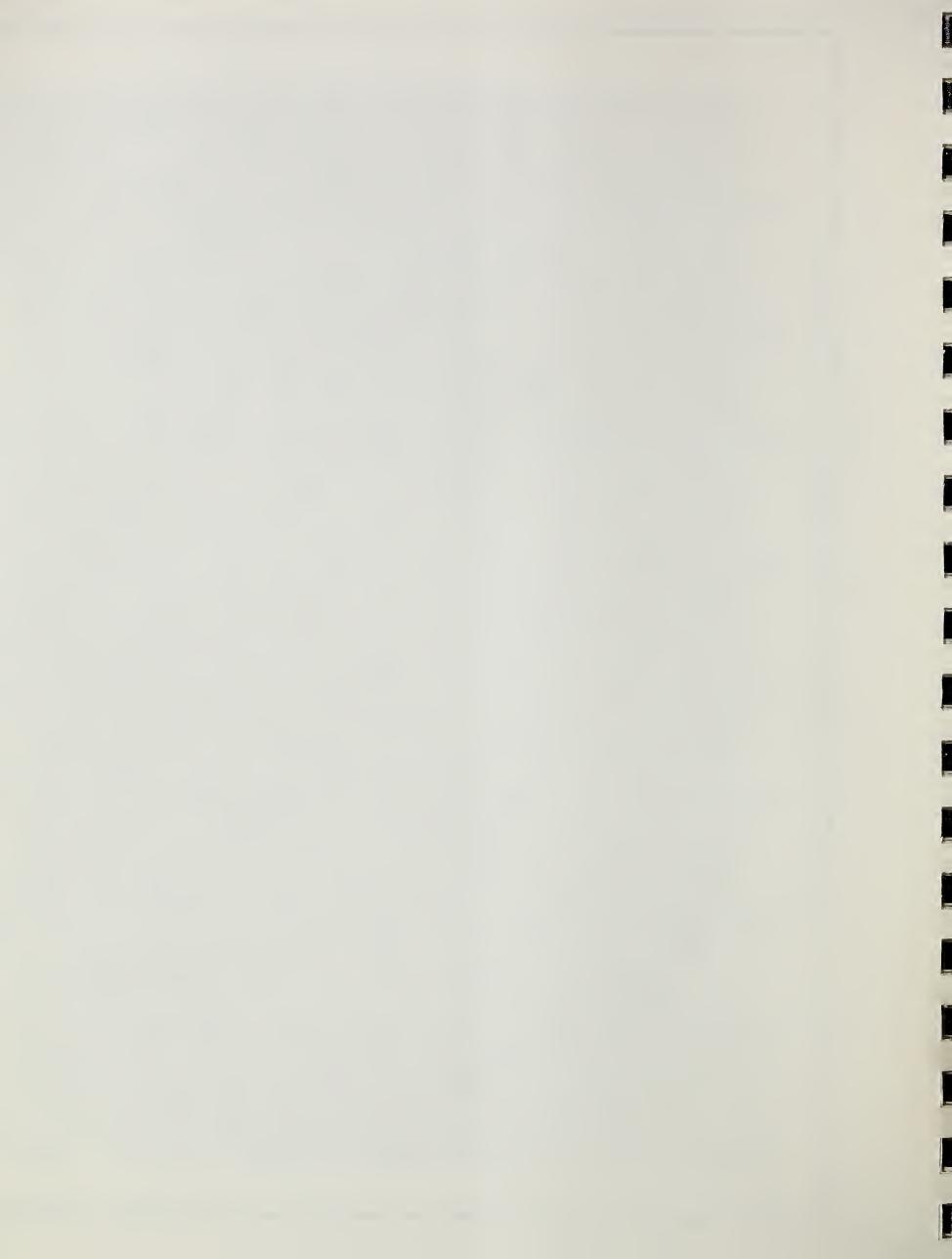


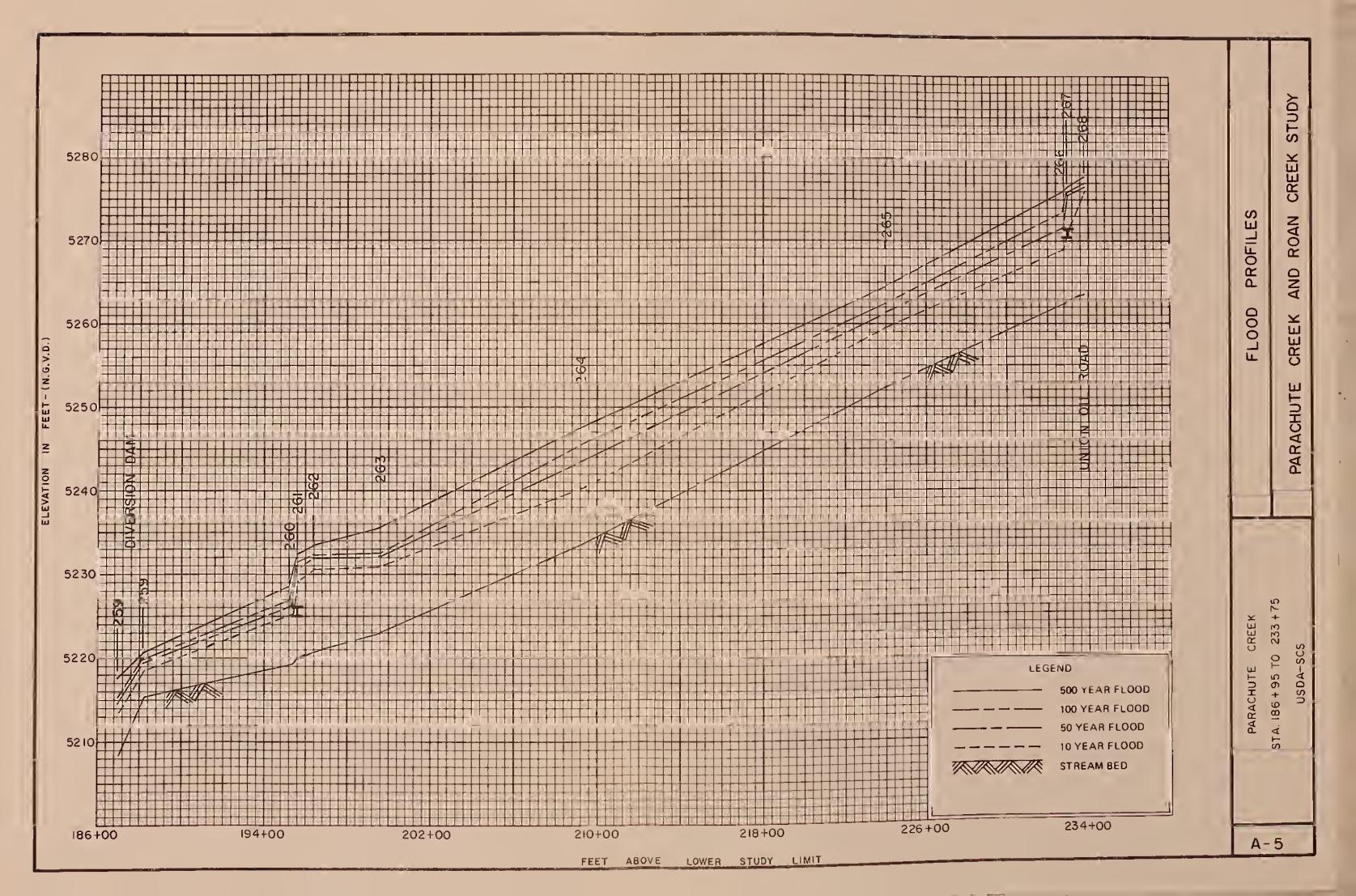


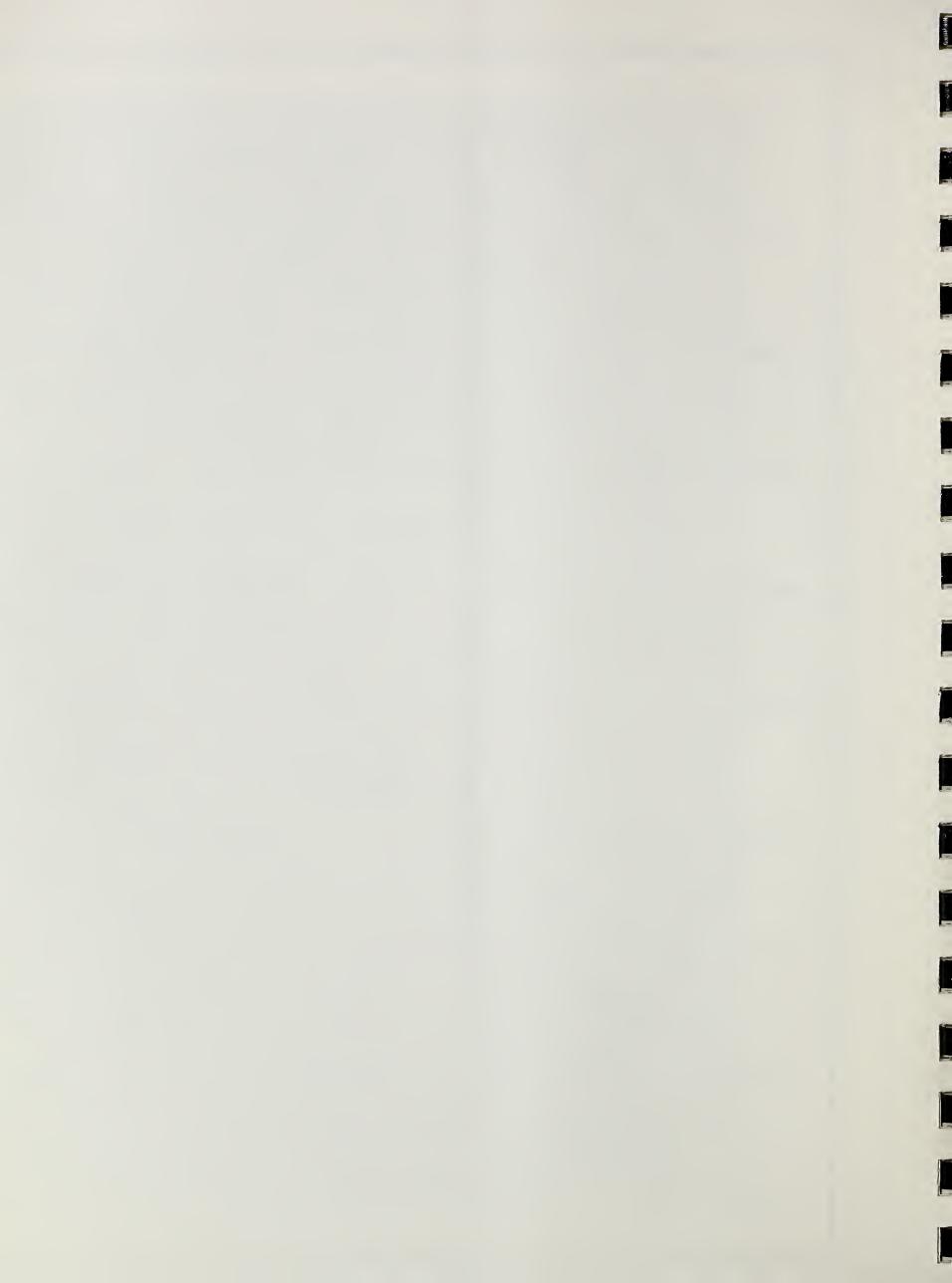


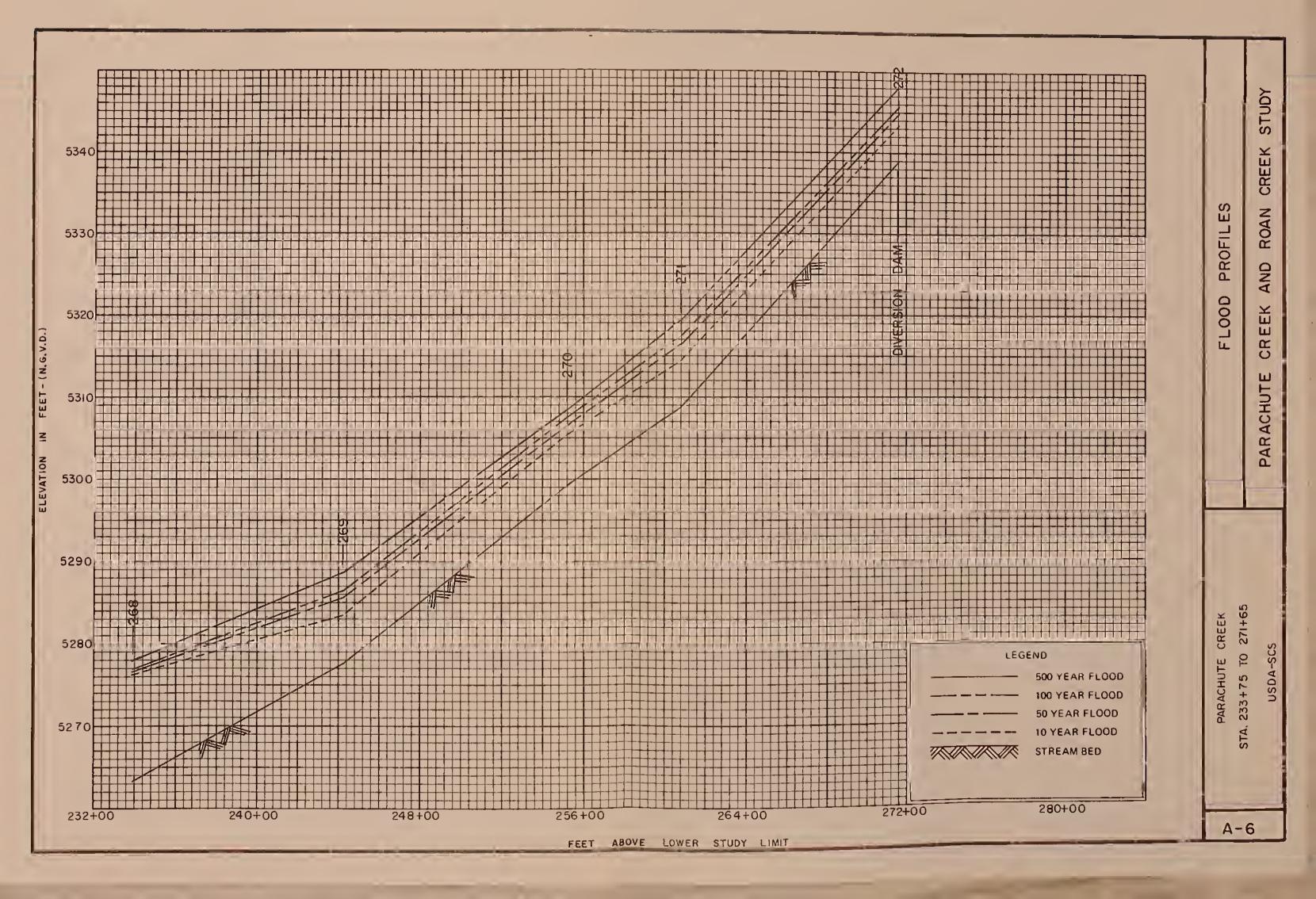




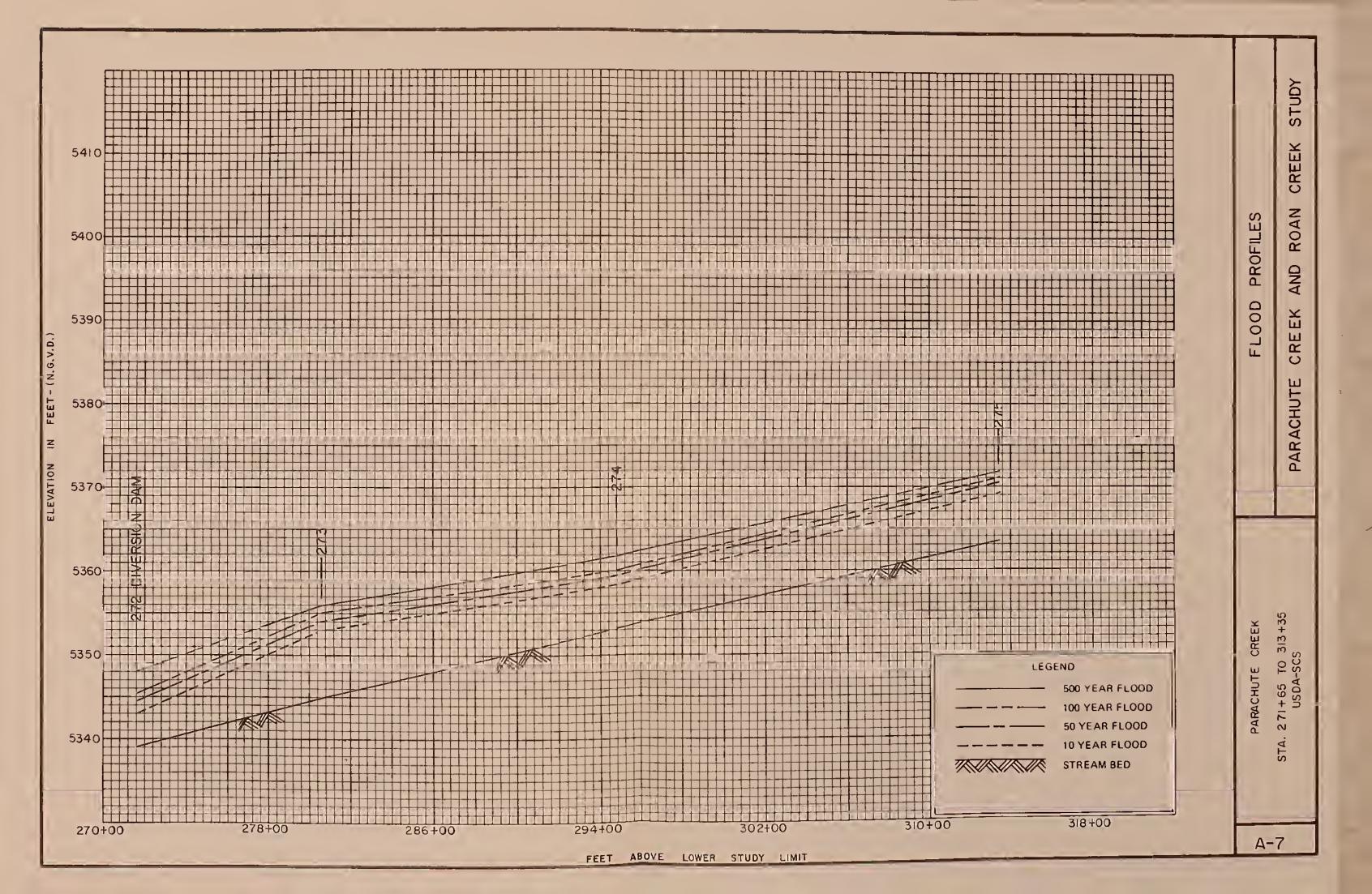




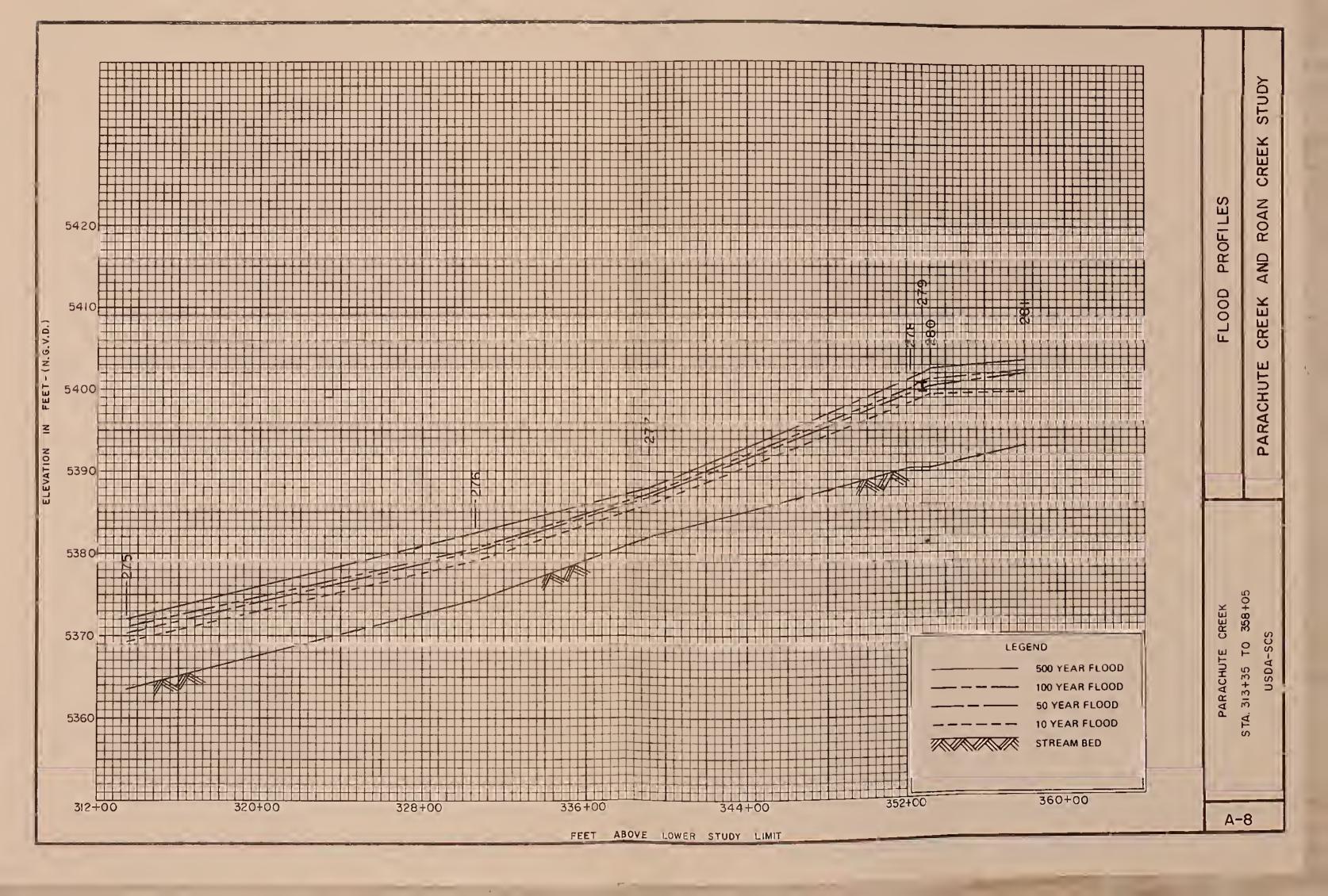




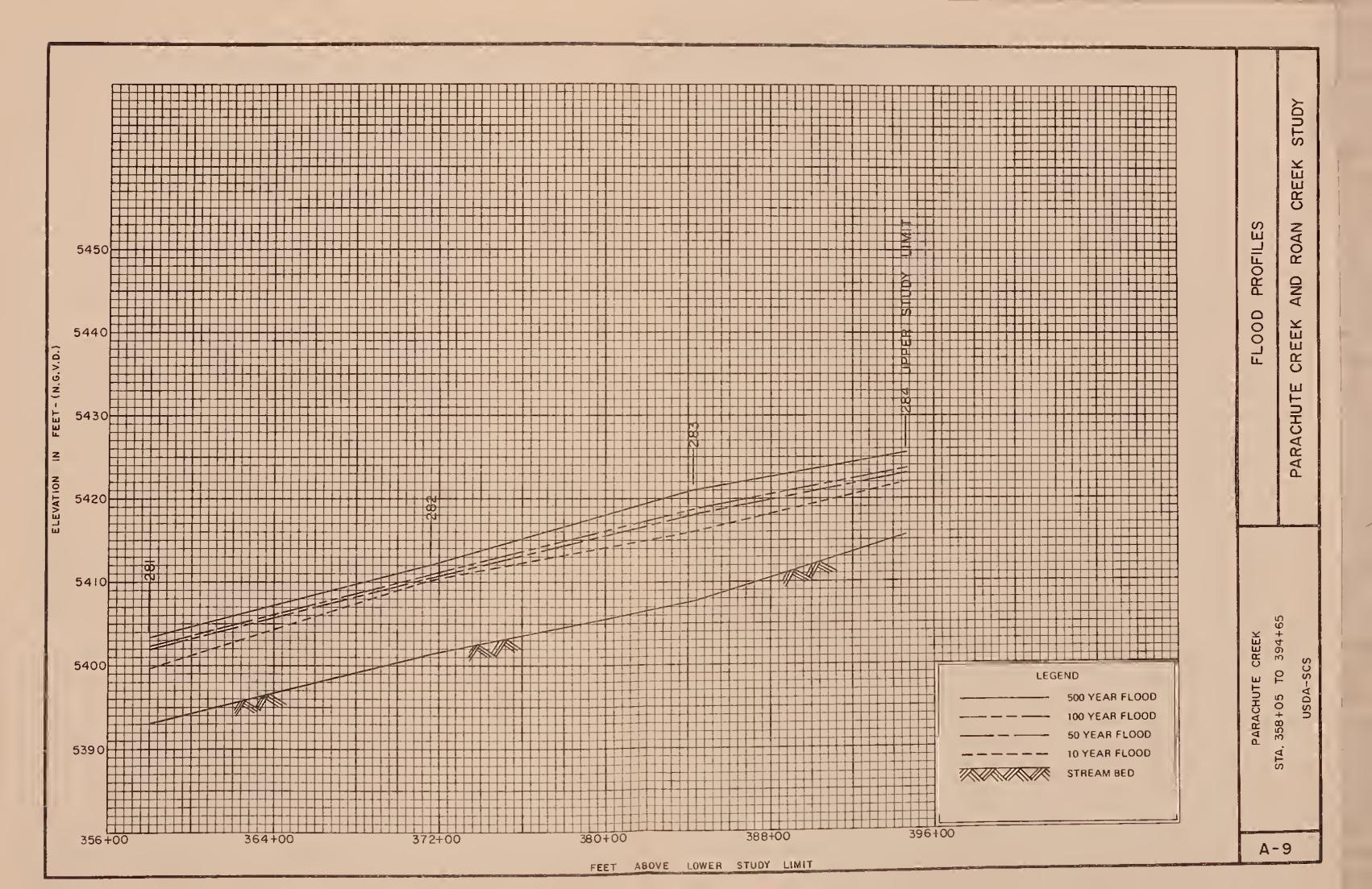




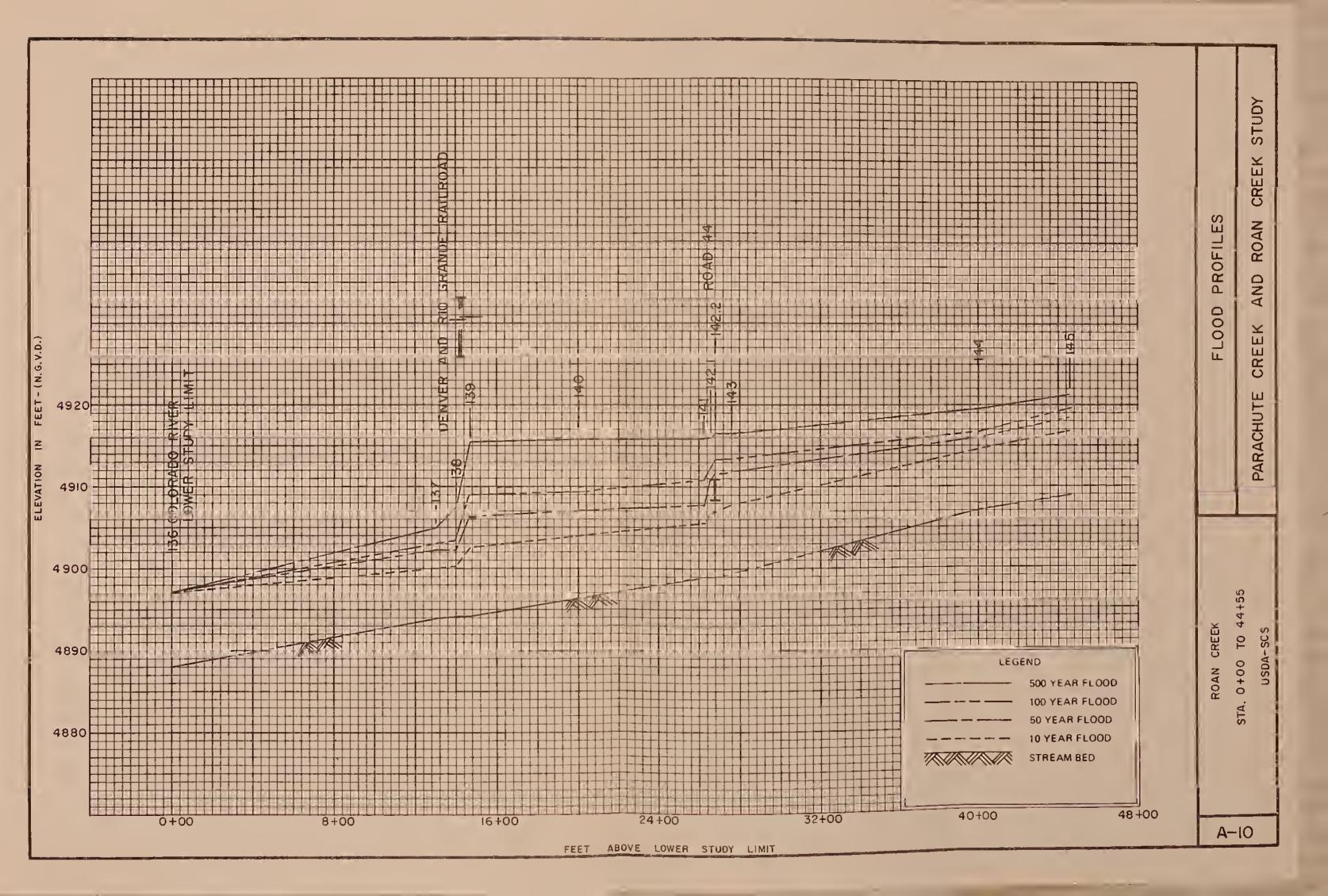


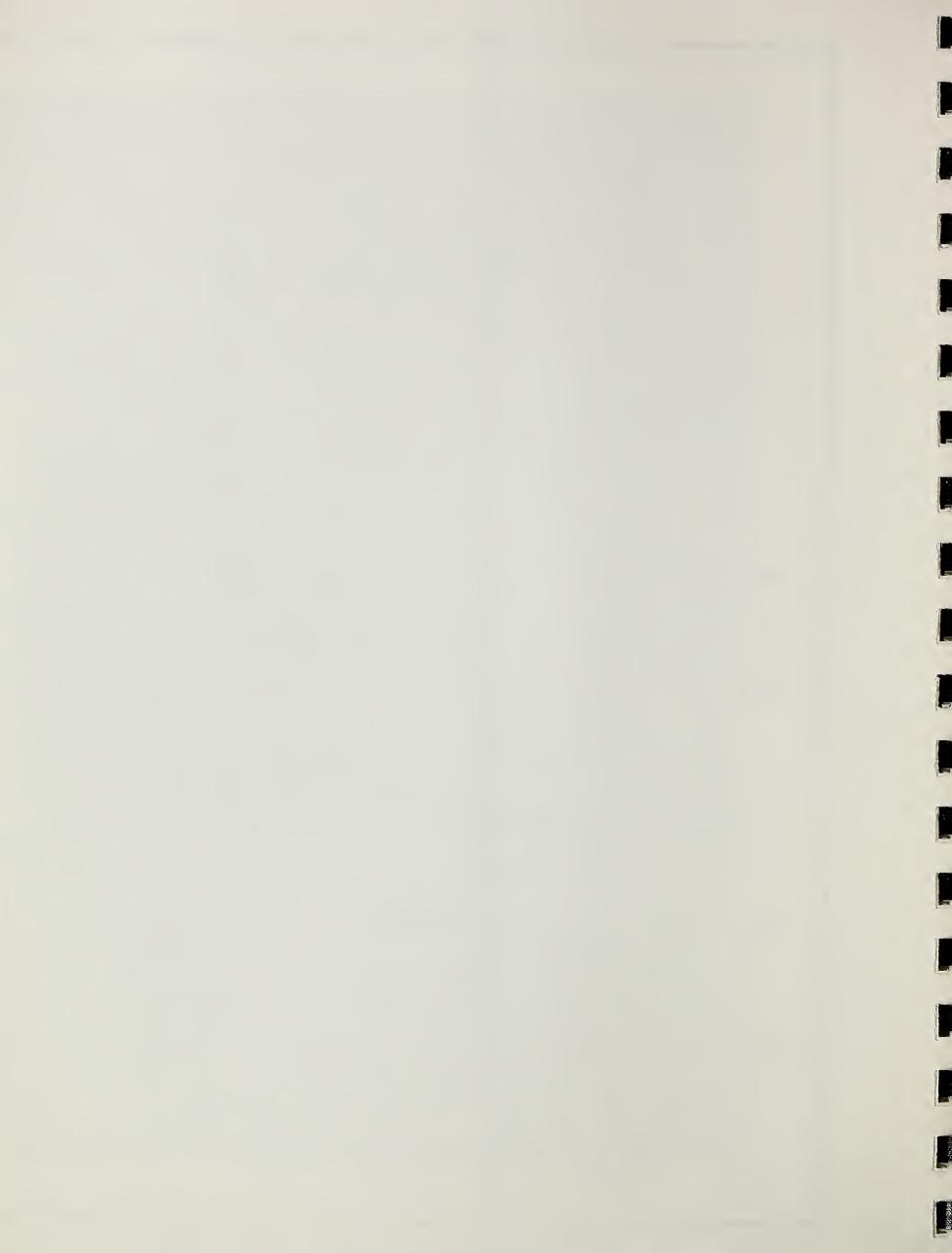


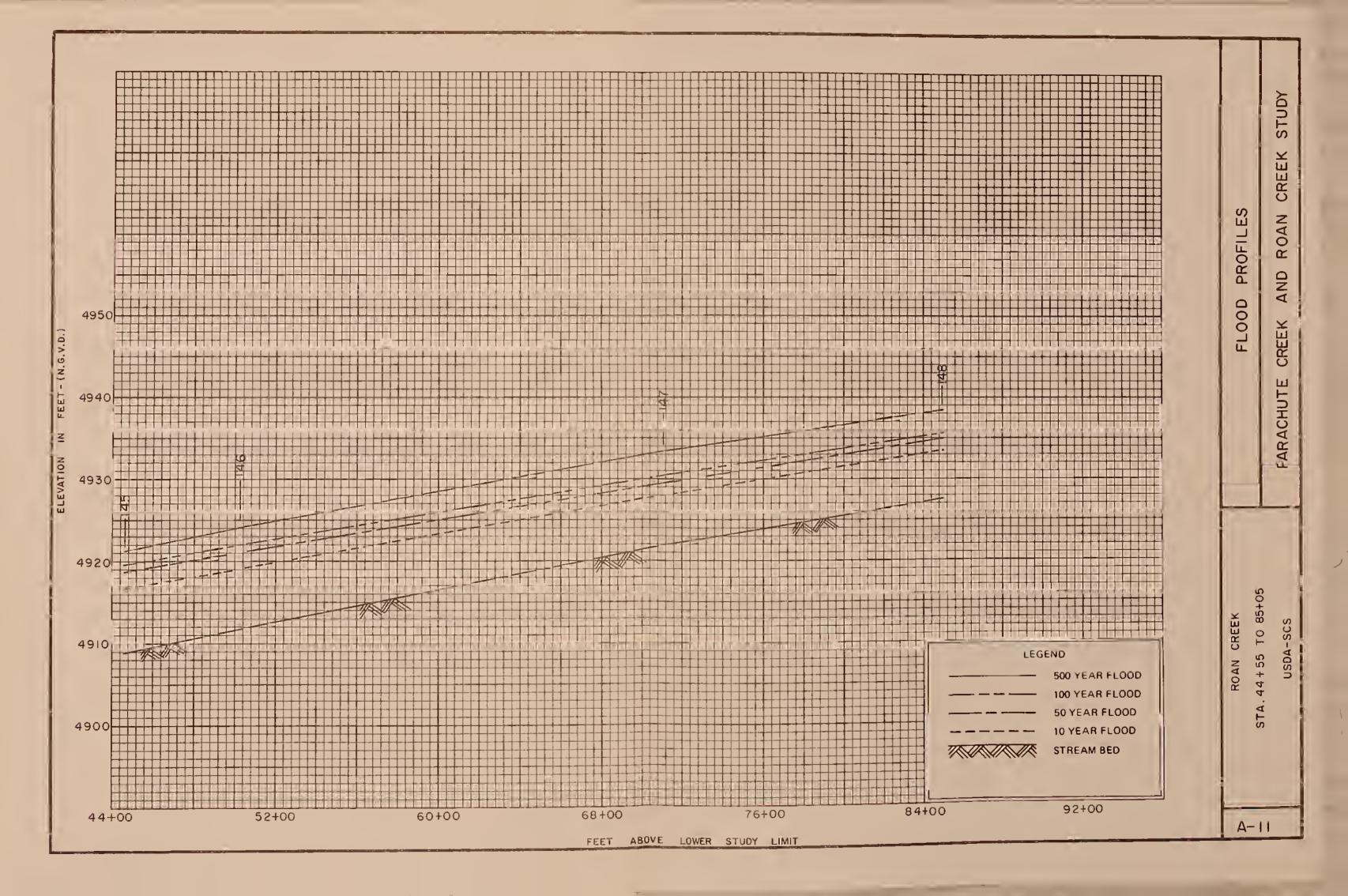




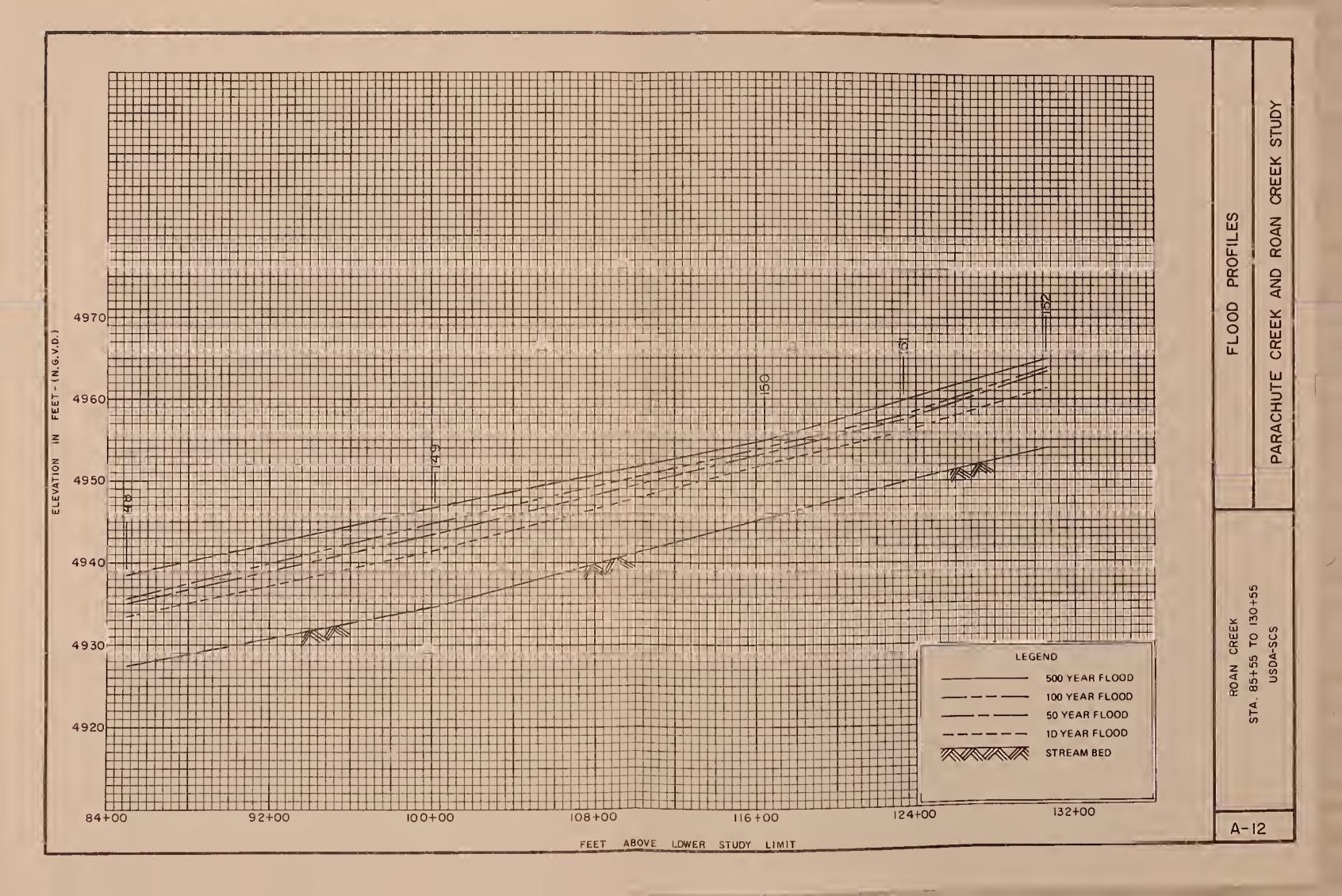




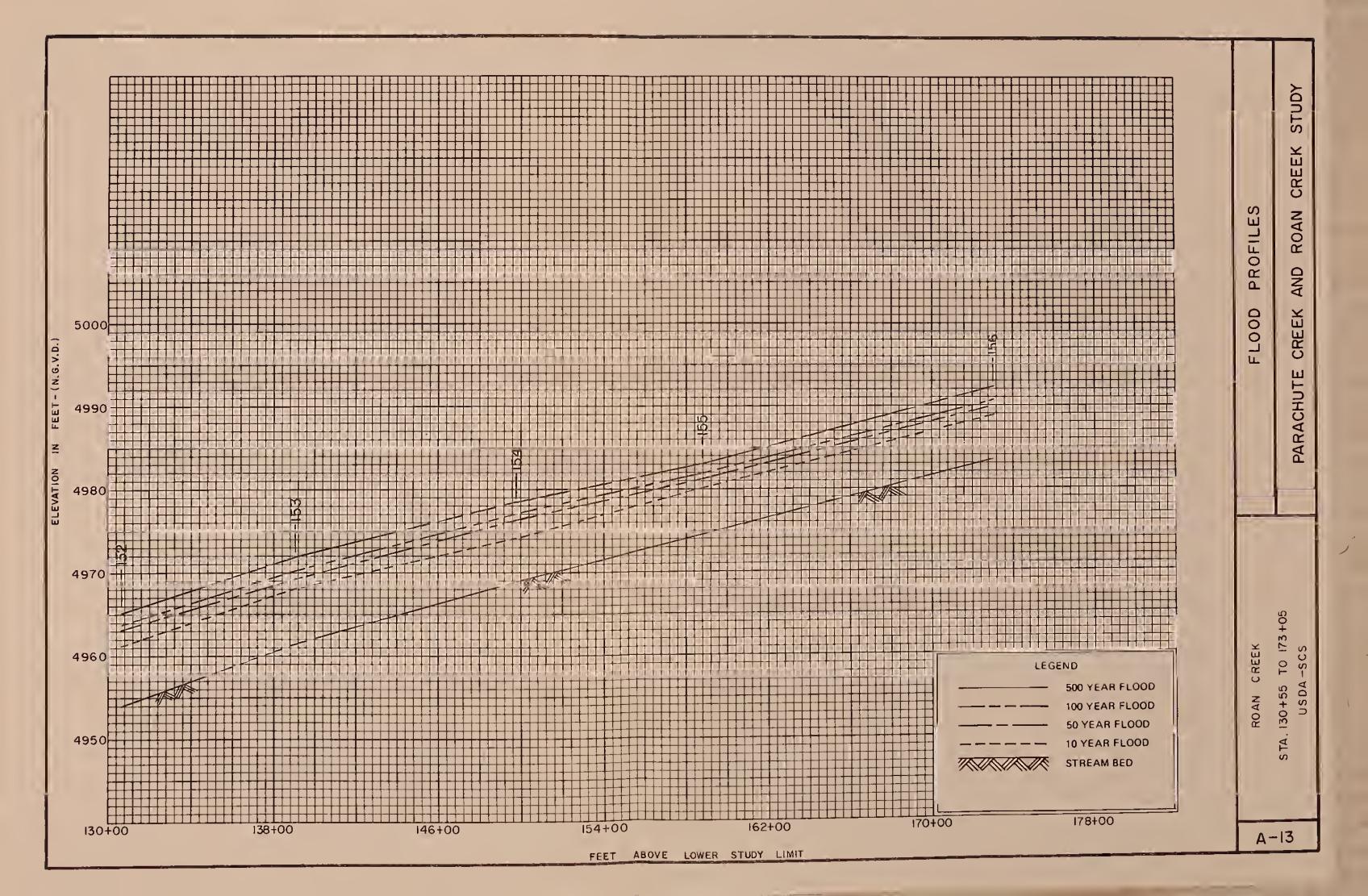




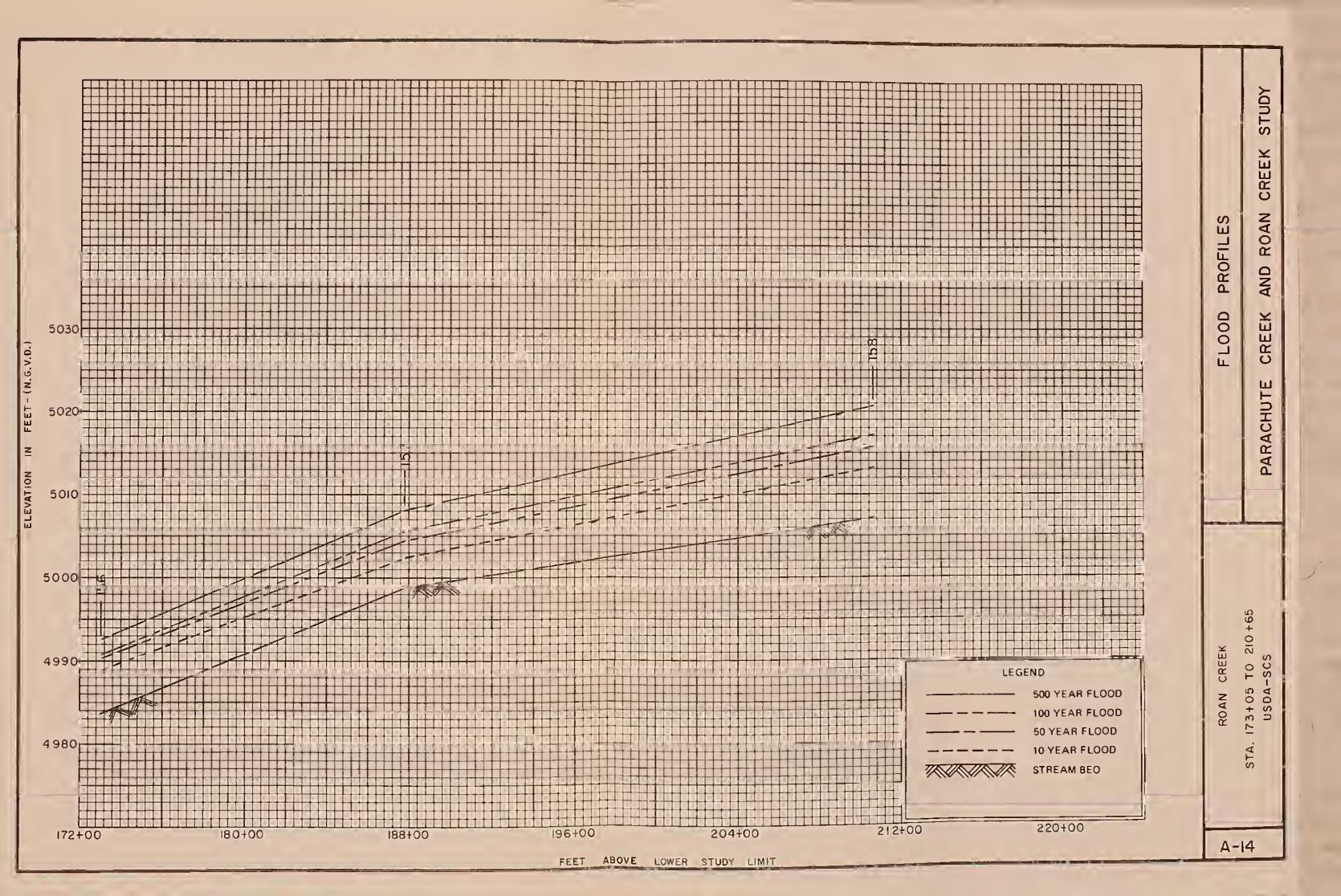




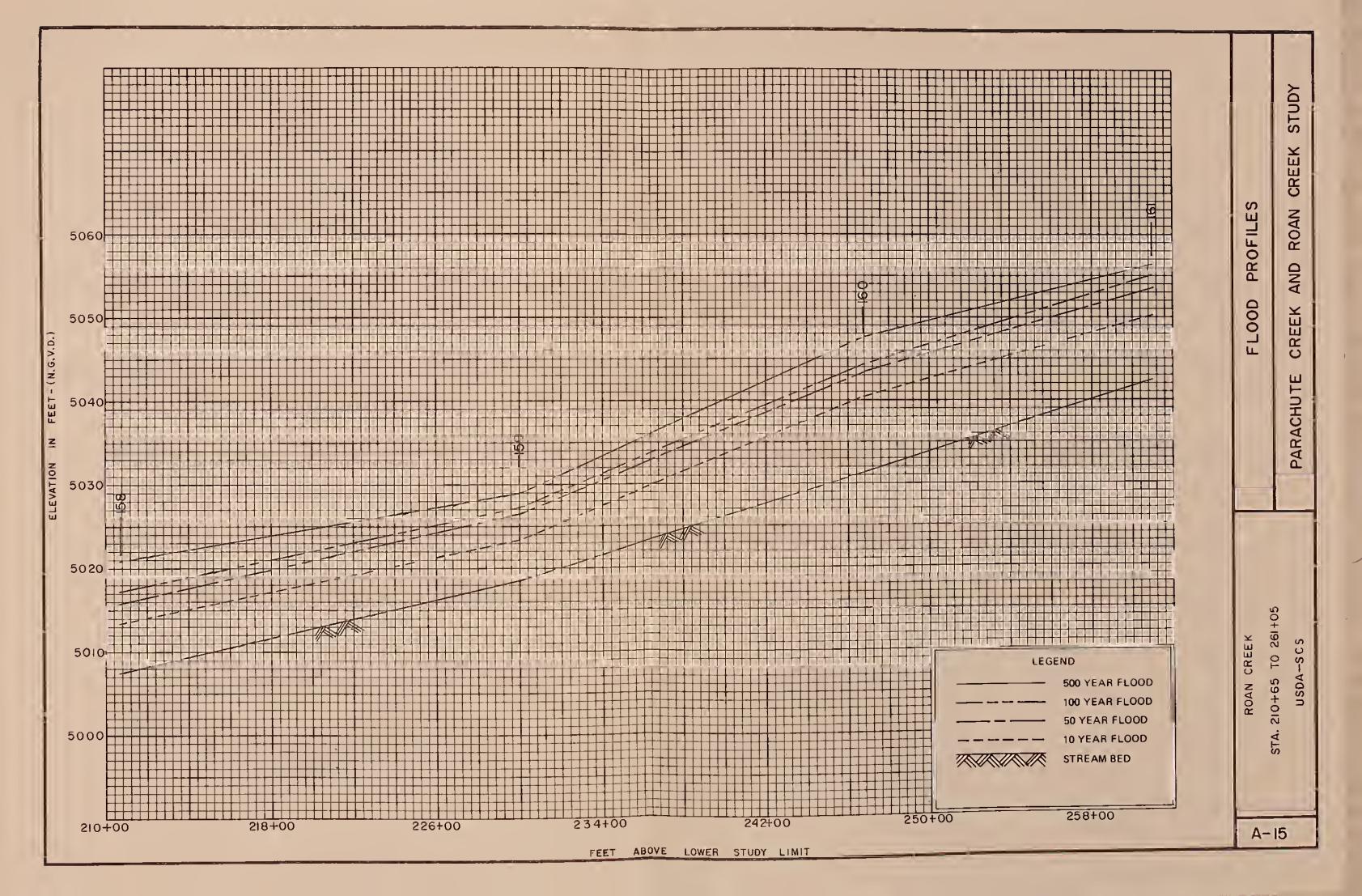




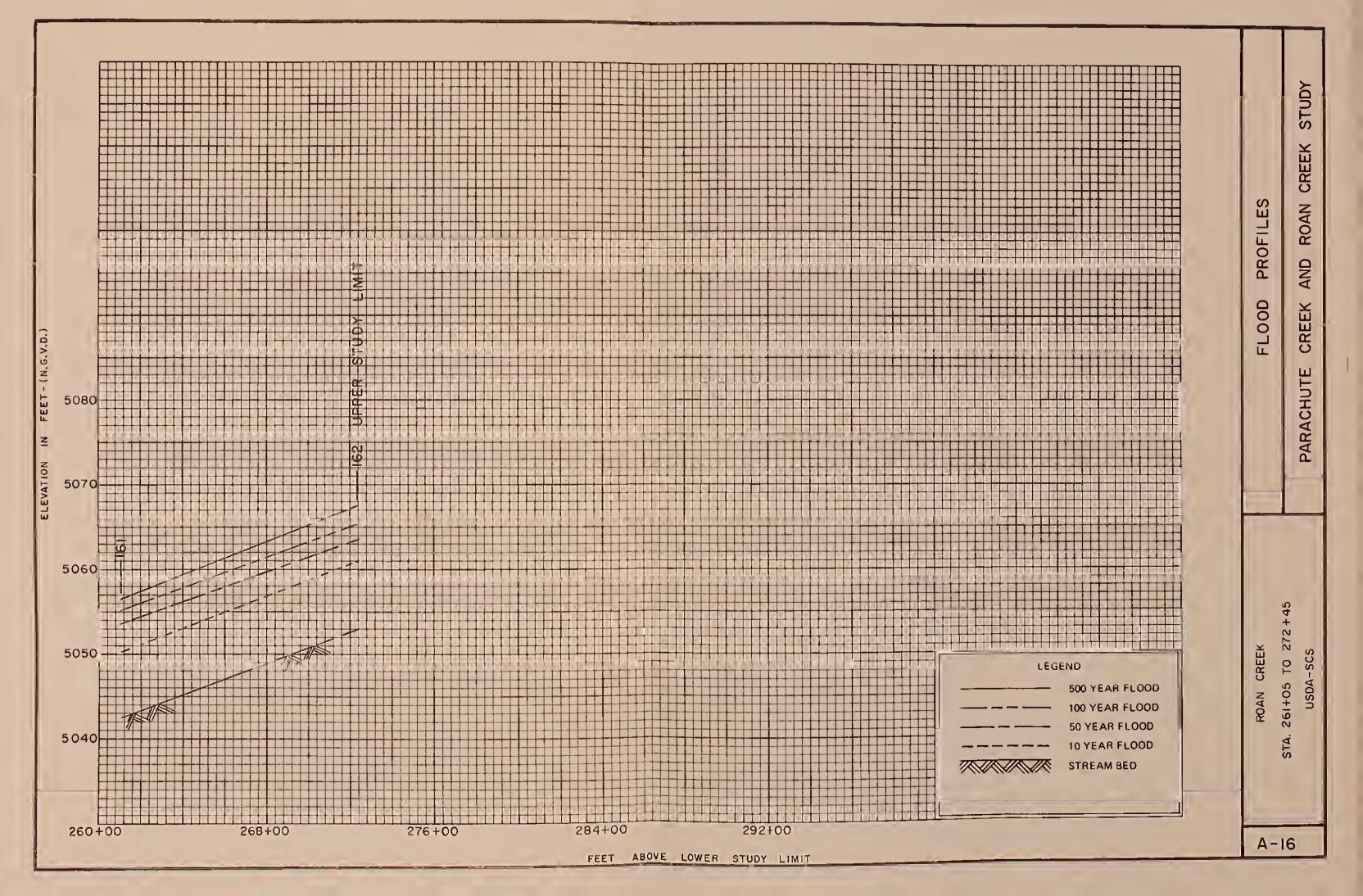




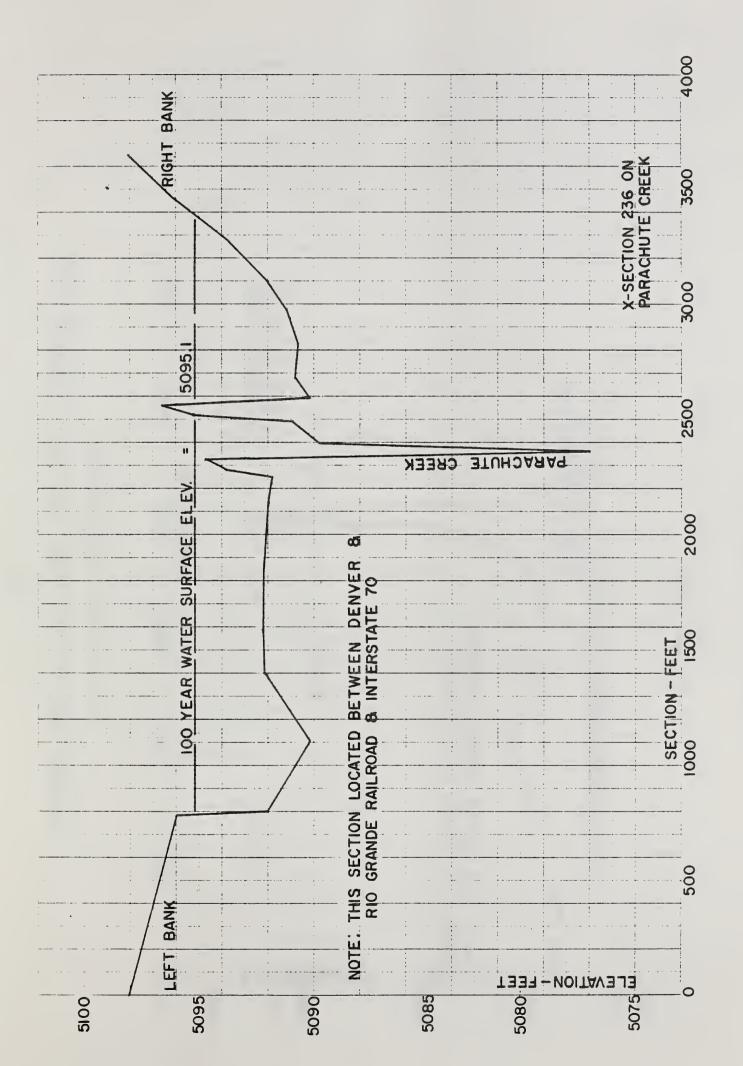






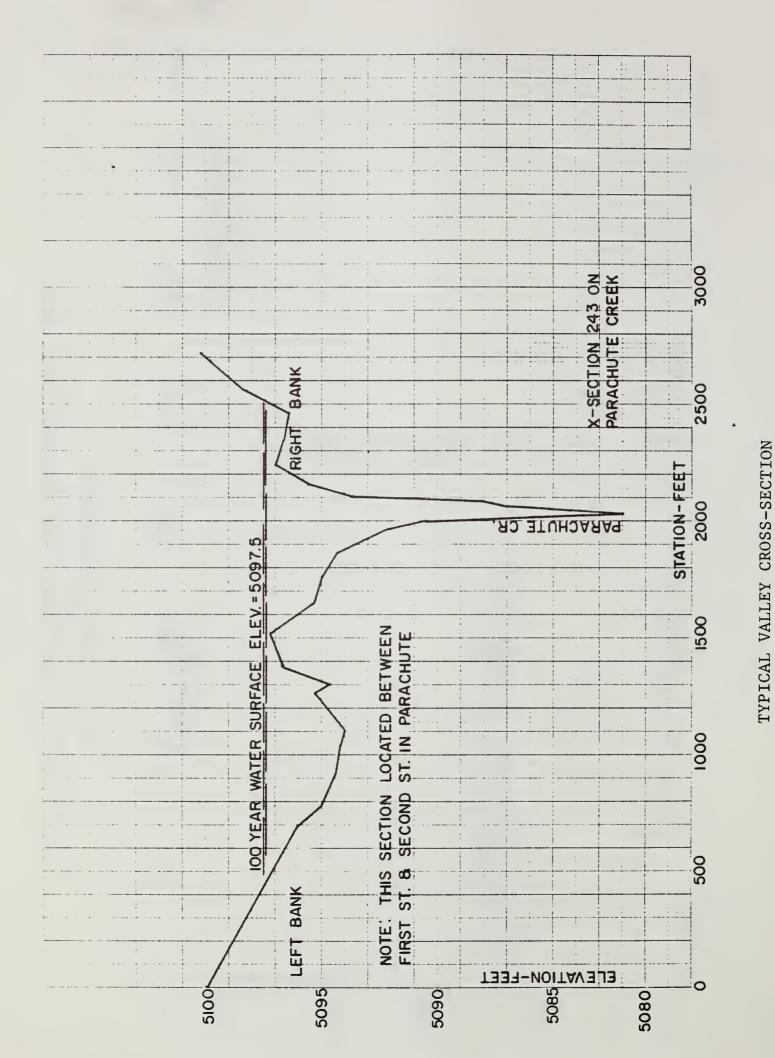




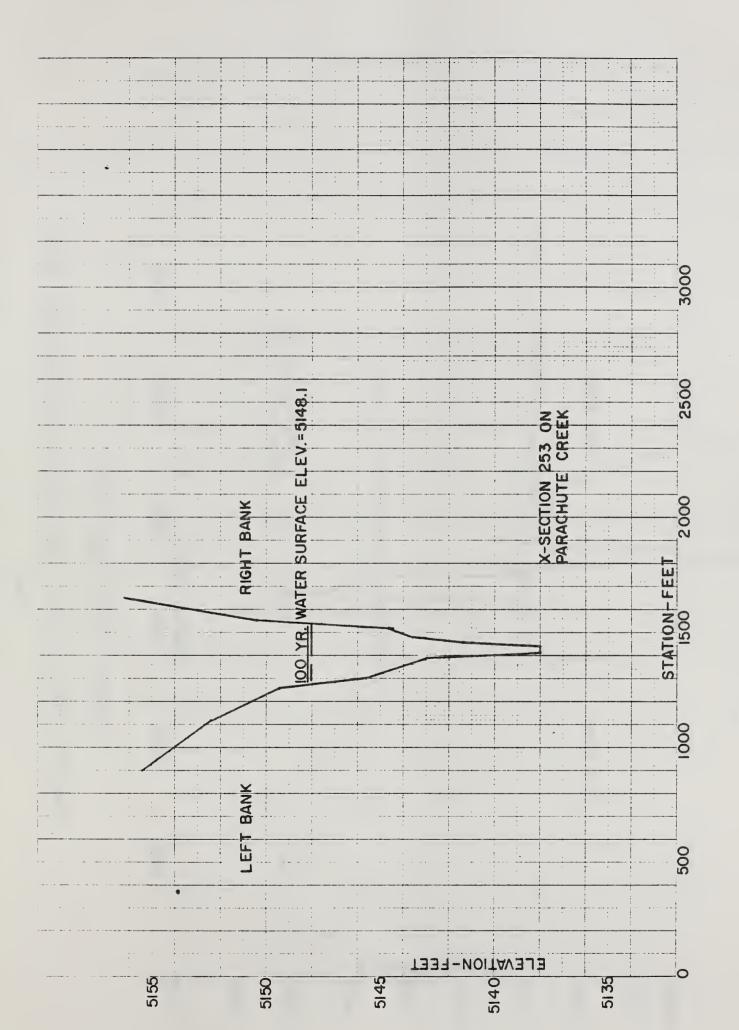


TYPICAL VALLEY CROSS-SECTION

PARACHUTE CREEK AND ROAN CREEK FLOOD PLAIN MANAGEMENT STUDY

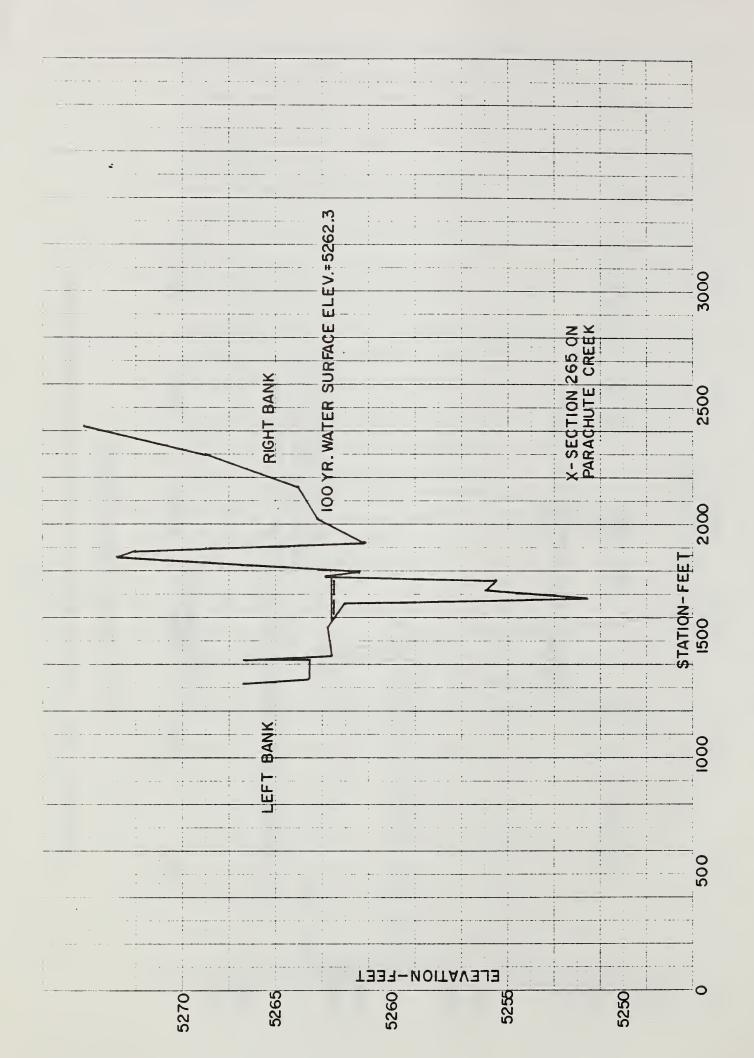


PARACHUTE CREEK AND ROAN CREEK FLOOD PLAIN MANAGEMENT STUDY



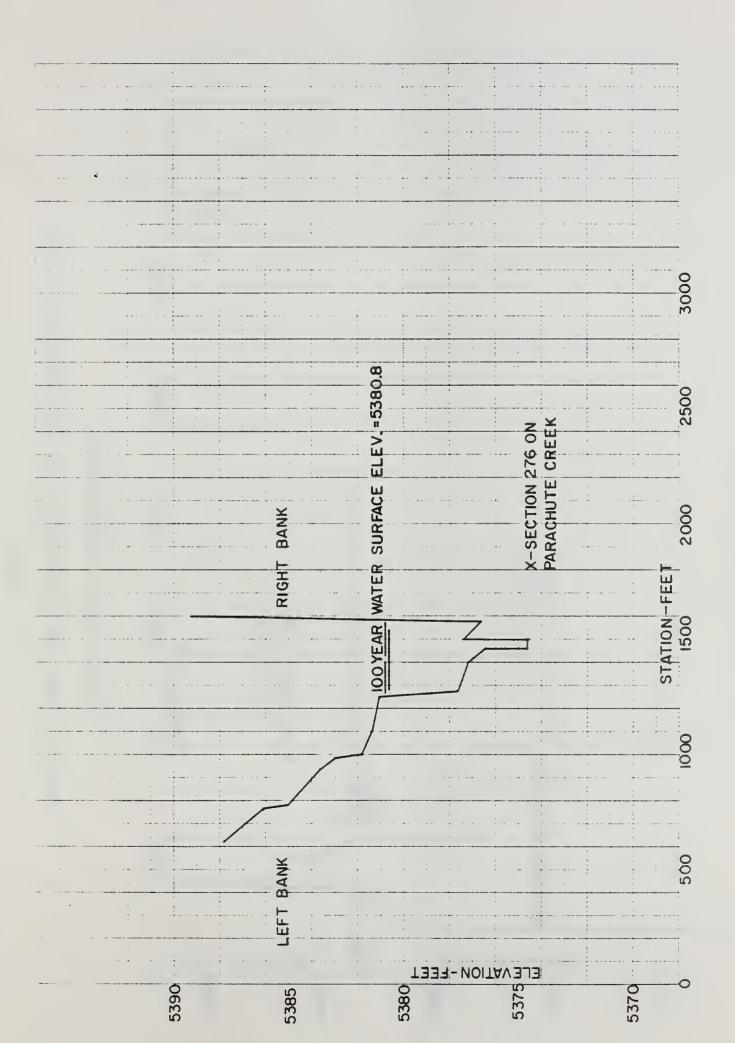
TYPICAL VALLEY CROSS-SECTION

PARACHUTE CREEK AND ROAN CREEK FLOOD PLAIN MANAGEMENT STUDY



TYPICAL VALLEY CROSS-SECTION

PARACHUTE CREEK AND ROAN CREEK FLOOD PLAIN MANAGEMENT STUDY



TYPICAL VALLEY CROSS-SECTION

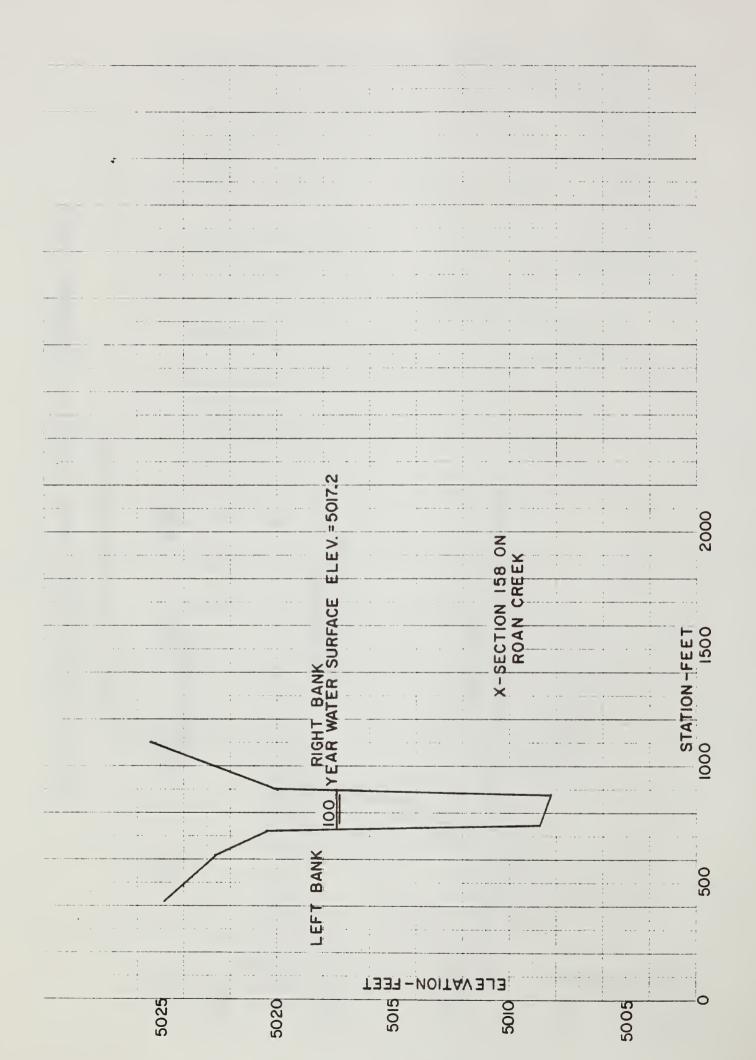
PARACHUTE CREEK AND ROAN CREEK FLOOD PLAIN MANAGEMENT STUDY

TYPICAL VALLEY CROSS-SECTION

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PARACHUTE CREEK AND ROAN CREEK FLOOD PLAIN MANAGEMENT STUDY

TYPICAL VALLEY CROSS-SECTION

Cross	Stationing from Mouth	Identification	Stream Bed Elevation	Crest-E	Crest-Elevation Feet Na Datum, and Peak D	Feet National Geodetic Vertical Peak Discharge c.f.s.	Vertical
Design- nation	Feet		Feet N.G.V.D.	10-Year Flood	50-Year Flood	100-Year Flood	500-Year Flood
226	00 + 0	Co. River at Parachute Cr.	5040.2	5053.8 35300 2/	5053.8 35300 2/	5053.8 35300 2/	5053.8 35300.2/
227	10 + 40		5048.6	5054.6 2350	5056.3	5056.4	5056.4
228	21 + 60	-	5056.9	5063.1 2350	5064.7 4250	5064.7 4400	5064.7
229	37 + 20		5065.3	5072.4 2350	5074.3 4250	5074.4	5074.4
230	38 + 15	County Road	5065.0	5073 . 9 2350	5075.7	5075.8 4400	5075.8
231	38 + 60		5066.2	5073.7 2350	5075.7	5075.8	5075.8
232	41 + 40		5067.6	5075.4	5077.0 4250	5077.1 4400	5077.1
233	09 + 97		5074.2	5080.2	5081.0 4250	5081.1 4400	5081.1

Flood elevations pertain to the primary channel and usually remain constant in a lateral direction across the flood plain. However, flood elevations in the outer portions of a cross section may differ from the primary channel due to road crossings, upstream diversions, etc.

^{2/} Dishcarge in Colorado River at 25-year frequency.

5077.0	Interstate-70 5077.0 Highway 5077.0	ate-70
5078.6	ate-70	
	Interstate-7(Highway Interstate-7Highway	10 45 80

Flood elevations pertain to the primary channel and usually remain constant in a lateral direction across the flood plain. However, flood elevations in the outer portions of a cross section may differ from the primary channel due to road crossings, upstream diversions, etc. 1/

Feer 10-Year 50-Year 100-Year 1	S	Stationing from Mouth	Identification	Stream Bed Elevation	Crest-E	Crest-Elevation Feet Na Datum, and Peak D	National Geodetic Discharge c.f.s.	Geodetic Vertical
5078.6 5088.7 5096.3 5097.3 1 5079.6 5088.8 5096.4 5600 1 5079.6 5089.4 5096.4 5097.3 1 5079.6 5089.4 5096.4 5097.3 1 5079.6 5089.4 5096.4 5097.3 1 5080.3 5089.4 5096.5 5097.4 1 5081.1 5089.4 5096.5 5097.4 1 5081.1 5091.0 5096.5 5097.4 1 5085.5 5097.4 5600 1 5081.1 5091.0 5096.5 5600 1 5085.8 5097.1 5600 1 5091.3 5102.8 1 5091.3 5102.3 5102.8 1	Feet			Feet N.G.V.D.	10-Year Flood	50-Year Flood	100-Year Flood	500-Year Flood
5079.6 5088.8 5096.4 5097.3 1 5079.6 5089.4 5096.4 5097.3 1 5079.6 5089.4 5096.5 5097.4 1 5080.3 5089.4 5096.5 5097.4 1 5081.0 5096.5 5097.4 1 5081.1 5091.0 5096.5 5097.4 1 5085.5 5097.8 1 5097.8 1 5091.3 5007.8 1 5600 1 5091.3 5100.4 5102.8 1 5091.3 5102.8 1 1	58 + 45			5078.6	5088.7 2350	5096.3	5097.3	5098.4
5079.6 5089.4 5096.4 5097.3 1 5079.6 5089.4 5096.5 5097.4 1 5080.3 5089.4 5096.5 5097.4 1 5081.1 5081.0 5096.5 5097.4 1 5085.5 5091.0 5096.6 5097.8 1 5081.3 5100.4 5102.3 5102.8 1 5091.3 5100.4 5102.3 5102.8 1	59 + 05			5079.6	5088.8 2350	5096.4	5097.3	5098.4
5089.4 5096.5 5097.4 1 2350 4400 5600 1 5089.4 5096.5 5097.4 1 2350 4400 5600 1 5091.0 5096.6 5600 1 2350 4400 5600 1 5100.4 5102.3 5600 1 2350 4400 5600 1 5350 4400 5600 1	60 + 20		First Street	5079.6	5089.4	5096.4	5097.3	5098.4
5089.4 5096.5 5097.4 2350 4400 5600 5091.0 5096.6 5097.5 2350 4400 5600 5095.8 5097.1 5097.8 5100.4 5102.3 5102.8 2350 4400 5600 5350 5600	60 + 55			5079.6	5089.4 2350	5096.5	5097.4	5098.6
5091.0 5096.6 5097.5 1 2350 4400 5600 1 5095.8 5097.1 5097.8 1 2350 4400 5600 1 5100.4 5102.3 5102.8 1 2350 4400 5600 1	60 + 65			5080•3	5089.4 2350	5096.5	5097.4	5098.6
5095.8 5097.1 5097.8 2350 4400 5600 5100.4 5102.3 5102.8 2350 4400 5600	62 + 30			5081.1	5091.0 2350	5096.6	5097.5	5098.6
5100.4 5102.3 5102.8 1 2350 4400 5600 1	68 + 70			5085.5	5095.8 2350	5097.1	5097.8	5099.1 10000
	75 + 25			5091.3	5100.4 2350	5102.3	5102.8 5600	5103.9

Flood elevations pertain to the primary channel and usually remain constant in a lateral direction across the flood plain. However, flood elevations in the outer portions of a cross section may differ from the primary channel due to road crossings, upstream diversions, etc. 1/

Cross	Stationing from Mouth	Identification	Stream Bed Elevation	Crest-E	Crest-Elevation Feet N Datum, and Peak D	National Geodetic Vertical Discharge c.f.s.	Vertical
Design- nation	Feet		Feer N.G.V.D.	10-Year Flood	50-Year Flood	100-Year Flood	500-Year Flood
246	85 + 25		5102.6	5108.6 2350	5109.6 4400	5110.4 5600	5112.0
247	91 + 05		5105.6	5113.3 2350	5115.0 4400	5115.5 5600	5116.8
248	99 + 75		5112.5	5118.9 2350	5121.1 4400	5122.2 5600	5126.7 10000
249	111 + 55		5127.7	5134.4 2350	5136.2 4400	5137.0 5600	5139.2
250	120 + 35		5132.4	5140.4 2350	5142.3 4400	5143.2 5600	5144.9
251.1	120 + 50	Private Road	5132.4	5140.5 2350	5142.4 4400	5143.3 5600	5145.8
251.2	120 + 65		5132.4	5143.0 2350	5144.5 4400	5145.0 5600	5146.3
252	121 + 05		5132.4	5142.8 2350	5144.5 4400	5145.0 5600	5146.5

Flood elevations pertain to the primary channel and usually remain constant in a lateral direction across the flood plain. However, flood elevations in the outer portions of a cross section may differ from the primary channel due to road crossings, upstream diversions, etc. ات

ertical	500-Year Flood	5149.6	5159.2	5174.4	5193.1	5208.1 10000	5217.8 10000	5220.8 10000	5228.8
National Geodetic Vertical Discharge c.f.s.	100-Year Flood	5148.1 5600	5157.9 5600	5170.7	5190.2	5206.1 5600	5215.3 5600	5219.9 5600	5227.1
Feer Peak	50-Year Flood	5147.2 4400	5157.6	5169.7	5189.3	5205.0 4400	5214.6 4400	5219.5 4400	5226.6
Crest-Elevation Datum, and	10-Year Flood	5145.2 2350	5156.7 2350	5167.3 2350	5187.3 2350	5202.5 2350	5213.3 2350	5218.7 2350	5225.3
Stream Bed Elevation	Feet N.G.V.D.	3.0	. 8	5.0	7.0	8.9	3.6	5.3	9.6
-	P. G.	5138.0	5151.8	5162.0	5180.4	5196.8	5208.6	Dam 5215.3	5219.6
Identification								Diversion Dam	
Stationing from Mouth	Feer	127 + 05	139 + 05	148 + 85	166 + 35	180 + 15	186 + 95	188 + 15	195 + 35
	Design- nation	253	254	255	256	257	258	259	260

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al	500-Year Flood	5232.3 0000	5233.7 0000	5235.6 0000	5247.3 9400	5264.9 9400	5276.0 9400	5276.5 9400	5277.8
Geoderic Verrical	500-Ye F1ood	5232 10000	5233 10000	5235 10000	5247 9400	5264 9400	5276 9400	5276 9400	5277
National Geoderi Discharge c.f.s.	100-Year Flood	5231.3 5600	5232.4 5600	5232.5 5600	5245.4 5300	5262.3 5300	5273.5 5300	5275.6 5300	5276.8
Crest-Elevation Feet N Datum, and Peak D	50-Year Flood	5230.8 4400	5232.1 4400	5232.2 4400	5243.5 4200	5261.6 4200	5272.7 4200	5275.3 4200	5276.4
Crest-E	10-Year Flood	5229.1 2350	5230.9 2350	5231.1 2350	5240.1 2200	5259.7 2200	5269.1 2200	5270.9 2200	5276.0
Stream Bed Elevation	Feer N.G.V.D.	5220.0	5220.8	5223.0	5233.7	5252.5	5262.8	5263.0	5263.4
Identification		County Road						Union Oil Road	
Stationing from Mouth	Feet	195 + 65	196 + 45	199 + 55	209 + 15	224 + 05	232 + 85	233 + 05	233 + 75
Cross	Design- nation	261	262	263	264	265	266	267	268

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Cross	Stationing from Mouth	Identification	Stream Bed Elevation	Crest-E	Crest-Elevation Feet Na Datum, and Peak D	National Geodetic Discharge c.f.s.	Vertical
Design- nation	Feer		Feet N.G.V.D.	10-Year Flood	50-Year Flood		500-Year Flood
269	244 + 15		5277.3	5283.2 2200	5285.3 4200	5286.4 5300	5288.2 9400
270	255 + 25		5299.5	5305.6	5306.7 4200	5307.2	5308.6 9400
271	260 + 75		5308.6	5314.4	5316.2	5317.4	5319.1 9400
272	271 + 65	Diversion Dam	5339.1	5343.1 2200	5344.6 4200	5345.3	5348.1 9400
273	280 + 65		5346.0	5353.2	5354.4	5355.0	5356.1 9400
274	294 + 75		5353.3	5358.8 2200	5359.7	5360.2	5361.8 9400
275	313 + 35		5363.7	5369.4	5370.6	5371.1	5372.2 9400
276	330 + 55		5374.6	5379.2	5380.3	5380.8	5382.3 9400

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Verrical	500-Year Flood	5388.0 9400	5401.5 9400	5401.9 9400	5402.6 9400	5402.9 9400	5403.7 9400	5412.1 9400	5421.0 9400
National Geodetic Vertical Discharge c.f.s.	100-Year Flood	5387.2 5300	5400.2 5300	5400.4 5300	5401.6 5300	5401.8 5300	5402.1 5300	5410.8 5300	5418.8
Crest-Elevation Feet Na Datum, and Peak Di	50-Year Flood	5386.8	5399.8 4200	5399.9	5401.5 4200	5401.7 4200	5402.0 4200	5410.5 4200	5418.1 4200
Crest-El	10-Year Flood	5386.0 2200	5398.6 2200	5398.6 2200	5398.6 2200	5399.4 2200	5399.8	5410.3 2200	5415.9
Stream Bed Elevation	Feet N.G.V.D.	5382.0	5390.3	5390.3	5390.3	5390.6	5393.0	5401.5	5407.6
Identification				Railroad & County Road					
Stationing from Mouth	Feec	339 + 15	352 + 15	352 + 30	352 + 75	353 + 25	358 + 05	371 + 65	384 + 25
Cross Section	Design- nation	277	278	279.1	279.2	280	281	282	283

Flood elevations pertain to the primary channel and usually remain constant in a lateral direction across the flood plain. However, flood elevations in the outer portions of a cross section may differ from the primary channel due to road crossings, upstream diversions, etc. 1

TABLE 1

FLOOD FREQUENCY-ELEVATION AND DISCHARGE DATA 1/

Vertical	500-Year Flood	5425.6 9400				-	
National Geodetic Vertical Discharge c.f.s.	100-Year Flood	5423.5 5300					
Crest-Elevation Feet National Geodeti Datum, and Peak Discharge c.f.s.	50-Year Flood	5422.9 4200				-	
Crest-E	10-Year Flood	5422.0 2200					
Stream Bed Elevation	Feet N.G.V.D.	5415.6	,				
Identification		Upper Study Limit on Parachute Cr.					
Stationing from Mouth	Feet	394 + 65		-			
Cross	Design- nation	284					

Flood elevations pertain to the primary channel and usually remain constant in a lateral direction across the flood plain. However, flood elevations in the outer portions of a cross section may differ from the primary channel due to road crossings, upstream diversions, etc. 1/

Geodetic Vertical	500-Year Flood	4897.1	4904.9	4907.6	4915.7	4915.9	4915.9 17500	4916.0 17500	4916.7
National Geodetic Discharge c.f.s.	100-Year Flood	4897.1 37000 2/	4903.0	4903.5 9700	490 8. 9 9700	4909.2 9700	4910.8 9700	4912.3 9700	4913.4
Crest-Elevation Feet Na Datum, and Peak Di	50-Year Flood	4897.1 37000 2/	4902.3	4902.4 7400	4906.5	4906.9	4907.9	4910.4	4911.7
Crest-El	10-Year Flood	4897.1 37000 2/	4900.2 3650	4901.5 3650	4902.7 3650	4903.5 3650	4905.3	4906.7 3650	4906.8
Stream Bed Elevation	Feer N.G.V.D.	4888.3	4893.9	4894.3	4894.1	4896.4	4898.8	4898.8	4898.8
Identification		Co. River at Roan Cr.		Denver & Rio Grande Railroad				44 Road	
Stationing from Mouth	Feer	00 + 0	13 + 00	13 + 90	14 + 55	20 + 05	26 + 25	26 + 65	26 + 85
Cross	Design- nation	136	137	138	139	140	141	142.1	142.2

Flood elevations pertain to the primary channel and usually remain constant in a lateral direction across the flood plain. However, flood elevations in the outer portions of a cross section may differ from the primary channel due to road crossings, upstream diversions, etc. 1/

2/ Discharge in Colorado River at 25-year frequency.

Geodetic Vertical	500-Year Flood	4916.7 17500	4919.3	4921.2 17500	4924.1 17500	4933.4	4938.6	4946.9	4955.1
National Geodeti Discharge c.f.s.	100-Year Flood	4913.4 9700	4916.7 9700	4919.5	4922.0 9700	4930.6	4935.6	4944.9	4954.1
Crest-Elevation Feet N Datum, and Peak I	50-Year Flood	4911.8 7400	4916.0 7400	4918.8 7400	4921.1 7400	4929.7 7400	4935.0 7400	4943.7 7400	4953.4
Crest-E	10-Year Flood	4907.8 3650	4914.7 3650	4916.9 3650	4919.1 3650	4928.0 3650	4933.7 3650	4941.3 3650	4952.0
Stream Bed Elevation	Feer N.G.V.D.	4899.2	4907.0	4908.9	4911.9	4921.9	4927.5	4934.8	4945.7
Identification									
Stationing from Mouth	Feer	27 + 55	39 + 95	44 + 55	50 + 25	71 + 05	85 + 05	100 + 25	116 + 45
Cross Section	Design- nation	143	144	145	146	147	148	149	150

Flood elevations pertain to the primary channel and usually remain constant in a lateral direction across the flood plain. However, flood elevations in the outer portions of a cross section may differ from the primary channel due to road crossings, upstream diversions, etc. 1/

Geodetic Vertical	ar 500-Year Flood	17500	4965.2	4972.1	4978.6	4983.2	4992.4	5008.0	5020.9
National Discharge	100-Year Flood	4958.1	4963.9	4970.2	4976.9	4981.8	4990.7	5005.5	5017.2
Crest-Elevation Feet Datum, and Peak	50-Year Flood	4957.6	4963.4	4969.5	4976.2	4981.2	4990.2 7400	5004.4	5015.9
Crest	10-Year Flood	4956.1 3650	4961.4 3650	4968.1 3650	4974.2	4980.2 3650	4988.9	5002.4 3650	5013.3
Stream Bed Elevation	Feet N.G.V.D.	4949.9	4954.1	4961.3	4968.9	4974.5	4983.6	4998.8	5007.3
Identification							-		
Stationing from Mouth	Feet	123 + 45	130 + 55	139 + 05	149 + 85	158 + 85	173 + 05	187 + 85	210 + 65
Cross	Design- nation	151	152	153	154	155	156	157	158

Flood elevations pertain to the primary channel and usually remain constant in a lateral direction across the flood plain. However, flood elevations in the outer portions of a cross section may differ from the primary channel due to road crossings, unstream diversions, etc. 1/

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Stationing from Mouth	oning	Identification	Stream Bed Elevation	Crest-E	Crest-Elevation Feet Na Datum, and Peak D	Feer National Geodetic Vertical Peak Discharge c.f.s.	Verrical
Feet			Feet N.G.V.D.	10-Year Flood	50-Year Flood	100-Year Flood	500-Year Flood
230 + 05			5018.5	5023.7 3650	5026.3 7400	5027.2 9700	5029.1 17500
246 + 85			5031.3	5040.3 3650	5043.2 7400	5044.2 9700	5047.7 17500
261 + 05			5042.7	5050.2	5053.8	5055.1 9700	5056.6
272 + 45		Upper Study Limit on Roan Cr.	5053.3	5061.0 3650	5063.8 7400	5065.3 9700	5067.5 17500
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Flood elevations pertain to the primary channel and usually remain constant in a lateral direction across the flood plain. However, flood elevations in the outer portions of a cross section may differ from the primary channel due to road crossings, upstream diversions, etc. 1-







